ICC-ES Evaluation Report

ESR-1008

Reissued April 2025 | *This report also contains:*
Revised April 24, 2025 | - **City of LA Supplement**
Subject to renewal April 2026 | - **CA Supplement w/ DSA and OSHPD**

DIVISION: 03 00 00—CONCRETE	REPORT HOLDER:	EVALUATION SUBJECT:
Section: 03 15 19—Cast-In Concrete Anchors Section: 03 16 00—Concrete Anchors	LEVIAT GMBH A CRH COMPANY	HALFEN HTA ANCHOR CHANNELS AND HS / HSR / HZS CHANNEL BOLTS

1.0 EVALUATION SCOPE

Compliance with the following codes:

- 2024, 2021, 2018, and 2015 *International Building Code® (IBC)*
- 2024, 2021, 2018, and 2015 *International Residential Code (IRC)*
- 2013 *Abu Dhabi International Building Code* (ADIBC)†

†The ADIBC is based on the 2009 IBC. 2009 IBC code sections referenced in this report are the same sections in the ADIBC.

Main references of this report are for the 2024 IBC and IRC. See Table 13 and Table 14 for applicable sections of the code for previous IBC and IRC editions.

Property evaluated:

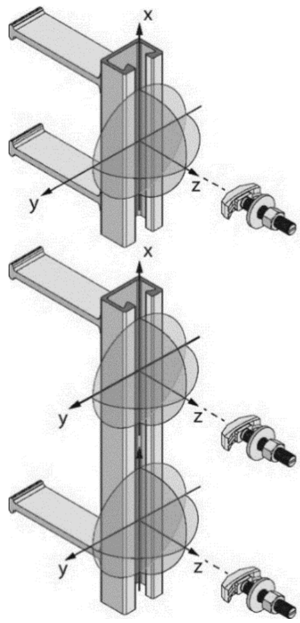
Structural

2.0 USES

HALFEN HTA anchor channels and HALFEN HS / HZS channel bolts are used as anchorage in concrete to resist static, wind, and seismic (IBC Seismic Design Categories A and B) tension loads (N_{ua}) and shear loads perpendicular to the longitudinal channel axis ($V_{ua,y}$), or any combination of these loads (as illustrated in Figure 1) applied at any location between the outermost anchors of the anchor channel.

HALFEN HTA anchor channel (HTA 40/22) and HALFEN HSR channel bolts are used as anchorage in concrete to resist static, wind and seismic (IBC Seismic Design Categories A through F) tension loads (N_{ua}) and shear loads perpendicular to the longitudinal channel axis ($V_{ua,y}$), shear loads acting in the direction of the longitudinal channel axis ($V_{ua,x}$) or any combination of these loads (as illustrated in Figure 1) applied at any location between the outermost anchors of the anchor channel.

The use is limited to cracked or uncracked normal-weight concrete having a specified compressive strength, f'_c, of 2,500 psi to 10,000 psi (17.2 MPa to 69.0 MPa) [minimum of 24 MPa is required under ADIBC Appendix L, Section 5.1.1]. The anchor channels are an alternative to anchors described in Section 1901.3 of the 2024 IBC. The anchor channels may also be used where an engineered design is submitted in accordance with Section R301.1.3 of the IRC.



tension load: z-direction (in direction of anchor)

shear load: y-direction (perpendicular to longitudinal axis of channel), x-direction (longitudinal to the channel axis; only HSR channel bolts)

FIGURE 1—LOAD DIRECTIONS

3.0 DESCRIPTION

3.1 Product information:

The HTA anchor channels consist of a C-shaped steel channel profile with round headed anchors (HTA 28/15, 38/17, 40/22, 41/22 and 50/30), I-shaped steel anchors (HTA 28/15, 38/17, 40/22, 50/30, 52/34, 55/42 and 72/48), T-shaped steel anchors (HTA 50/30, 52/34 and 55/42), or deformed reinforcing bars (HTA 40/22, 50/30, 52/34 and 55/42). Round headed anchors are forged to the channel back. I- and T-shaped anchors and deformed reinforcing bars are welded to the channel back (as illustrated in Figure 21 of this report). The maximum number of anchors per channel is not limited. The HALFEN HTA anchor channels are made of carbon steel channel profiles, or stainless steel (HTA 28/15, 38/17, and 41/22 only) channel profiles. The appropriate channel bolts are placed in the anchor channel. The anchor channels are shown in Figure 20 of this report. The available channel bolts feature either a hammer-head or a hook-head and are shown in Figure 22. Installation information and parameters are shown in Tables 10 through 12 of this report. The combination of the HALFEN HTA anchor channels and the corresponding HS, HZS and HSR channel bolts covered by this report are described in Table 2 of this report.

3.2 Material information:

Steel specifications for the channel profiles, anchors and channel bolts are given in Table 9 of this report.

3.3 Concrete:

Normal-weight concrete shall comply with Sections 1903 and 1905 of the IBC.

4.0 DESIGN AND INSTALLATION

4.1 General:

The design strength of anchor channels under the 2024 IBC, must be determined in accordance with ACI 318-19 chapter 17, and this report.

4.1.1 Determination of forces acting on anchor channel: Anchor channels shall be designed for critical effects of factored loads as determined by elastic analysis taking into account the elastic support by anchors and the partial restraint of the channel ends by concrete compression stresses. As an alternative, the triangular load distribution method in accordance with Section 4.1.2 through 4.1.4 to calculate the tension and shear loads on anchors shall be permitted. Design of adjacent anchor channels shall be in accordance with Section 4.1.6.

4.1.2 Tension loads: The tension loads, $N^a_{ua,i}$, on an anchor due to a tension load, N_{ua}, acting on the channel shall be computed in accordance with Eq.(17.2.1.2.1.a). An example for the calculation of the tension loads acting on the anchors is given in Figure 2.

$$N^a_{ua,i} = k \cdot A'_i \cdot N_{ua} \qquad (17.2.1.2.1.a)$$

where

A'_i = ordinate at the position of the anchor i assuming a triangle with the unit height at the position of load N_{ua} and the base length 2 ℓ_{in} with ℓ_{in} determined in accordance with Eq. (D-0.c). An example is provided in Figure 2.

$$k = 1/\sum A'_i \qquad (17.2.1.2.1.b)$$

$$\ell_{in} = 4.93 \cdot (I_y)^{0.05} \cdot \sqrt{s} \geq s\text{, in.} \qquad (17.2.1.2.1.c)$$

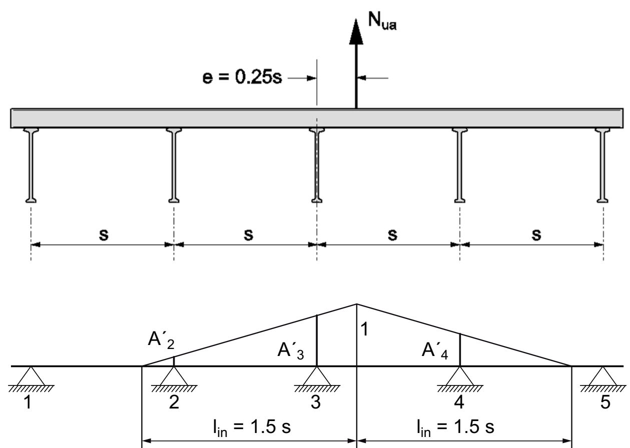
$$\ell_{in} = 13 \cdot (I_y)^{0.05} \cdot \sqrt{s} \geq s\text{, mm} \qquad (17.2.1.2.1.c)$$

s = anchor spacing, in. (mm)

N_{ua} = factored tension load on anchor channel, lbf (N)

I_y = the moment of inertia of the channel shall be taken from Table 1 of this report.

If several tension loads are simultaneously acting on the channel, a linear superimposition of the anchor forces for all loads shall be assumed. If the exact position of the load on the channel is not known, the most unfavorable loading position shall be assumed for each failure mode (e.g. load acting over an anchor for the case of failure of an anchor by steel rupture or pull-out and load acting between anchors in the case of bending failure of the channel).



$$A'_2 = \frac{(\ell_{in} - s - e)}{\ell_{in}} = \frac{0.25s}{\ell_{in}} = \frac{1}{6} \qquad N^a_{ua,1} = N^a_{ua,5} = 0$$

$$A'_3 = \frac{(\ell_{in} - e)}{\ell_{in}} = \frac{1.25s}{\ell_{in}} = \frac{5}{6} \qquad N^a_{ua,2} = \frac{1}{6} \cdot \frac{2}{3} \cdot N_{ua} = \frac{1}{9} \cdot N_{ua}$$

$$A'_4 = \frac{(\ell_{in} - s + e)}{\ell_{in}} = \frac{0.75s}{\ell_{in}} = \frac{1}{2} \qquad N^a_{ua,3} = \frac{5}{6} \cdot \frac{2}{3} \cdot N_{ua} = \frac{5}{9} \cdot N_{ua}$$

$$k = \frac{1}{A'_2 + A'_3 + A'_4} = \frac{2}{3} \qquad N^a_{ua,4} = \frac{1}{2} \cdot \frac{2}{3} \cdot N_{ua} = \frac{1}{3} \cdot N_{ua}$$

FIGURE 2—EXAMPLE FOR THE CALCULATION OF ANCHOR FORCES IN ACCORDANCE WITH THE TRIANGULAR LOAD DISTRIBUTION METHOD FOR AN ANCHOR CHANNEL WITH FIVE ANCHORS.

THE INFLUENCE LENGTH IS ASSUMED AS ℓ_{in} = 1.5s

4.1.3 Bending moment: The bending moment $M_{u,flex}$ on the channel due to tension loads acting on the channel shall be computed assuming a simply supported single span beam with a span length equal to the anchor spacing.

4.1.4 Shear loads:

4.1.4.1 Shear perpendicular to the channel axis: The shear load $V^a_{ua,y,i}$ on an anchor due to a shear load $V_{ua,y}$ acting on the channel perpendicular to its longitudinal axis shall be computed in accordance with Section 4.1.2 replacing N_{ua} in Eq. (17.2.1.2.1.a) by $V_{ua,y}$.

4.1.4.2 Shear longitudinal to the channel axis: The shear load $V^a_{ua,x,i}$ on an anchor due to a shear load $V_{ua,x}$ acting on the channel in direction of the longitudinal channel axis shall be computed as follows:

For the verification of the strength of the anchor channel for failure of the anchor or failure of the connection between anchor and channel, pryout failure and concrete edge failure in case of anchor channels arranged parallel to the edge without corner effects, the shear load $V_{ua,x}$ shall be equally distributed to all anchors for anchor channels with not more than three anchors or to three anchors for anchor channels with more than three anchors (as illustrated in Figure 3). The shear load $V_{ua,x}$ shall be distributed to those three that result in the most unfavorable design condition (in the example given in Figure 3 the shear load $V_{ua,x}$ shall be distributed to the anchors 10 to 12).

For verification of the strength of the anchor channel for concrete edge failure in case of anchor channels arranged perpendicular to the edge and in case of anchor channels arranged parallel to the edge with corner effects, the shear load $V_{ua,x}$, shall be equally distributed to all anchors for anchor channels with not more than three anchors, or to the three anchors closest to the edge or corner for anchor channels with more than three anchors (see Figure 4).

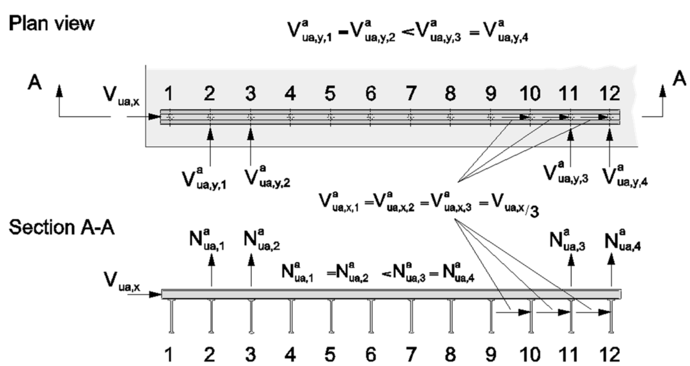
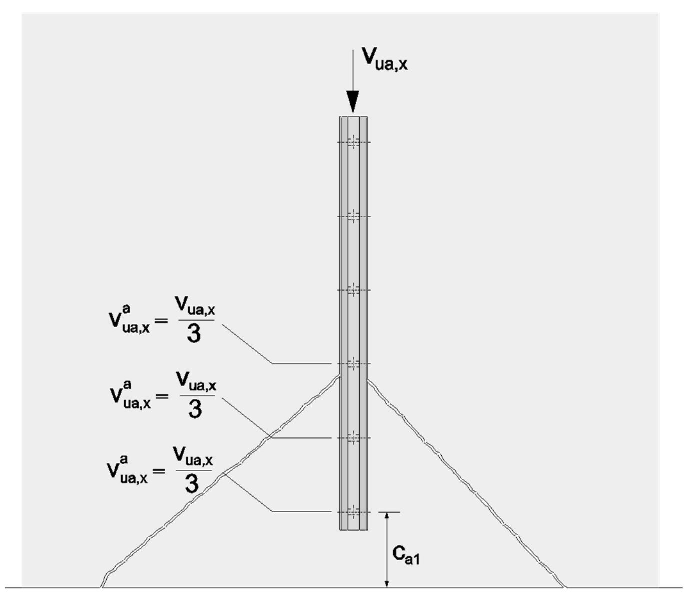


FIGURE 3—EXAMPLE FOR THE CALCULATION OF ANCHOR FORCES IN CASE OF ANCHOR CHANNELS WITH 12 ANCHORS LOADED IN SHEAR LONGITUDINAL TO THE CHANNEL AXIS FOR STEEL AND PRYOUT FAILURE



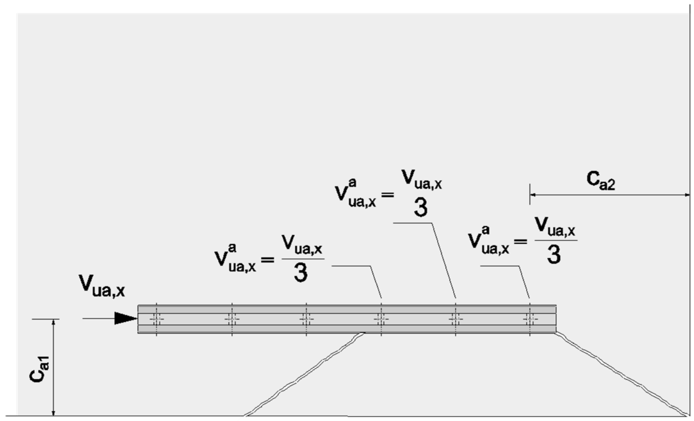


FIGURE 4—EXAMPLE FOR THE CALCULATION OF ANCHOR FORCES IN CASE OF ANCHOR CHANNELS WITH 6 ANCHORS LOADED IN SHEAR LONGITUDINAL TO THE CHANNEL AXIS FOR CONCRETE EDGE FAILURE

4.1.5 Forces related to anchor reinforcement: If tension loads are acting on the anchor channel, the factored tension forces of the anchor reinforcement for one anchor shall be computed for the factored tension load, $N^{a}_{ua,i}$, of the anchor assuming a strut- and-tie model.

If a shear load $V_{ua,y}$ is acting on the anchor channel, the resultant factored tension force of the anchor reinforcement $N_{ua,re}$, shall be computed by Eq.(17.2.1.2.1.d).

$N_{ua,re} = V_{ua,y}((e_s / z)+1)$, lbf (N) (17.2.1.2.1.d)

where (as illustrated in Figure 5):

e_s = distance between reinforcement and shear force acting on the fixture, in. (mm)

z = internal lever arm of the concrete member, in. (mm)

z = $0.85\, h'$

= $0.85\,(h - h_{ch} - 0.5\, d_a)$

$\leq \min \begin{Bmatrix} 2h_{ef} \\ 2c_{a1} \end{Bmatrix}$

h' see Figure 5

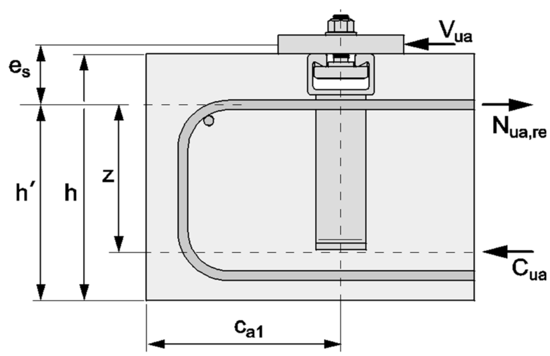
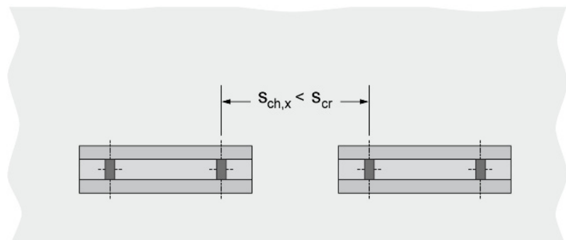
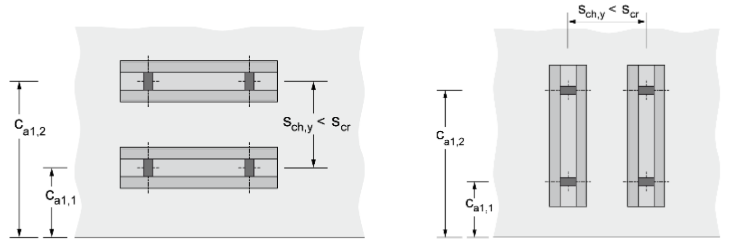


FIGURE 5—ANCHOR REINFORCEMENT TO RESIST SHEAR LOADS

4.1.6 Adjacent anchor channels: Anchor channels may be arranged as shown in Figure 6. Adjacent anchor channels must be of same size and consist of anchors with same type and embedment depth. In case of anchor channel configurations according to Figure 6b) and 6c) loaded in shear in any direction, the load shall be transferred to the adjacent anchor channels by a single plate (Figure 7).



a) Anchor channels in linear arrangement



b) Anchor channels arranged parallel to edge (n_1=2)

c) Anchor channels arranged perpendicular to edge(n_1=2)

FIGURE 6—INCLUDED CONFIGURATIONS OF ADJACENT ANCHOR CHANNELS

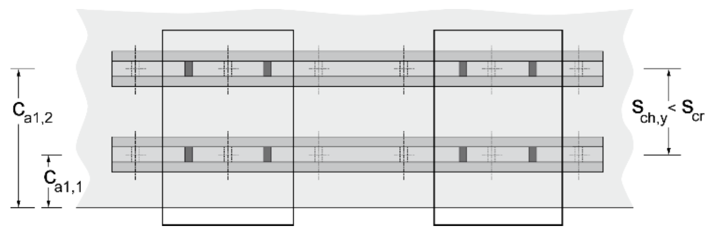


FIGURE 7—PERMISSIBLE CONFIGURATION WITH MULTIPLE ATTACHMENTS (n_1 = 2); TWO PLATES SHOWN. SHEAR TRANSFER BETWEEN ADJACENT ANCHOR CHANNELS BY ADJACENT PLATES.

4.2 Strength design:

4.2.1 General: The design strength of anchor channels under the 2024 IBC as well as Section R301.1.3 of the 2024 IRC shall be determined in accordance with ACI 318-19 Chapter 17 and this report.

Design parameters in this report and references to ACI 318 are based on the 2024 IBC (ACI 318-19) unless noted otherwise in Section 4.2.1 through 4.2.5 of this report.

The strength design shall comply with ACI 318-19 17.5.1.2 except as required in ACI 318-19 17.10.

Design parameters are provided in Tables 1 through 9 of this report. Strength reduction factors, ϕ, as given in ACI 318-19 17.5.3. and the tables of this report shall be used for load combinations calculated in accordance with Section 1605.1 of the 2024 IBC or Section 5.3 of ACI 318-19, as applicable.

In Table 17.5.2 (ACI 318-19) ϕN_n and ϕV_n are the lowest design strengths determined from all appropriate failure modes. ϕN_n is the lowest design strength in tension of an anchor channel determined from consideration of ϕN_{sa}, ϕN_{sc}, ϕN_{sl}, ϕN_{ss}, $\phi M_{s,flex}$, ϕN_{cb}, (anchor channels without anchor reinforcement to take up tension loads) or ϕN_{ca} (anchor channels with anchor reinforcement to take up tension loads), ϕN_{pn} and ϕN_{sb}. $\phi V_{n,y}$ is the lowest design strength in shear perpendicular to the axis of an anchor channel as determined from $\phi V_{sa,y}$, $\phi V_{sc,y}$, ϕV_{ss}, $\phi V_{ss,M}$, $\phi V_{sl,y}$, $\phi V_{cb,y}$ (anchor channels without anchor reinforcement to take up shear loads perpendicular to the channel axis), or $\phi V_{ca,y}$ (anchor channels with anchor reinforcement to take up shear loads perpendicular to the channel axis) and $\phi V_{cp,y}$. $\phi V_{n,x}$ is the lowest design strength in shear acting longitudinal to the channel axis of an anchor channel as determined from $\phi V_{sa,x}$, $\phi V_{sc,x}$, ϕV_{ss}, $\phi V_{ss,M}$, $\phi V_{sl,x}$, $\phi V_{cb,x}$, (anchor channel without anchor reinforcement to take up shear loads), or $\phi V_{ca,x}$ (anchor channel with anchor reinforcement to take up shear loads) *and* $\phi V_{cp,x}$. The design strength for all anchors of an anchor channel shall be determined.

4.2.2 Tension loads:

4.2.2.1 General: Following verifications are required:

a) Steel failure: Steel strength of anchor, strength of connection between anchor and channel, strength for local failure of channel lip, strength of channel bolt, bending strength of channel, see Section 4.2.2.2.

b) Concrete breakout strength of anchor in tension, see Section 4.2.2.3.

c) Pullout strength of anchor channel in tension, see Section 4.2.2.4.

d) Concrete side-face blow-out strength of anchor channels in tension, see Section 4.2.2.5.

4.2.2.2 Steel strength in tension: The nominal strength, N_{sa}, of a single anchor shall be taken from Table 3 of this report.

The nominal strength, N_{sc}, of the connection between anchor and channel profile shall be taken from Table 3 of this report.

The nominal strength of the channel lips to take up tension loads transmitted by a channel bolt, N_{sl}, shall be taken from Table 3 of this report. This value is valid only if the center-to-center distance between the channel bolts under consideration and adjacent channel bolts, s_{chb}, is at least $2b_{ch}$. If this requirement is not met, then the value N_{sl} given in Table 3 shall be reduced by the factor:

$$\frac{1}{1+\sum_{i=2}^{n+1}\left[\left(1-\frac{s_{chb,i}}{2b_{ch}}\right)^{2}\cdot\frac{N_{ua,i}^{b}}{N_{ua,1}^{b}}\right]} \qquad (17.6.1.3.3)$$

where

The center-to-center spacing between channel bolts shall not be less than 3 times the bolt diameter, d_s.

b_{ch} = channel width, taken from Table 1, in. (mm)

The nominal strength of the channel bolt, N_{ss}, shall be taken from Table 7 of this report.

The nominal bending strength of the anchor channel, $M_{s,flex}$, shall be taken from Table 3 of this report.

4.2.2.3 Concrete breakout strength in tension: The nominal concrete breakout strength, N_{cb}, of a single anchor in tension of an anchor channel shall be determined in accordance with Eq. (17.6.2.7.1)

$N_{cb} = N_b \cdot \psi_{s,N} \cdot \psi_{ed,N} \cdot \psi_{co,N} \cdot \psi_{c,N} \cdot \psi_{cp,N}$, lbf (N) (17.6.2.7.1)

Where anchors consist of deformed reinforcing bars and the minimum spacing requirement in Table 1 is met, verification for concrete breakout is not required provided that the reinforcing bars are lap sliced with reinforcing bars in the member according to the requirements of ACI 318-19 Section 25.5.

The basic concrete breakout strength of a single anchor in tension in cracked concrete, N_b, shall be determined in accordance with Eq. (17.6.2.7.2a).

$N_b = 24 \cdot \lambda \cdot \alpha_{ch,N} \cdot (f'_c)^{0.5} \cdot h_{ef}^{1.5}$, lbf (17.6.2.7.2a)

$N_b = 10 \cdot \lambda \cdot \alpha_{ch,N} \cdot (f'_c)^{0.5} \cdot h_{ef}^{1.5}$, N (17.6.2.7.2a)

where

$\lambda = 1$ (normal-weight concrete)

$\alpha_{ch,N} = (h_{ef} / 7.1)^{0.15} \leq 1.0$, (inch-pound units) (17.6.2.7.2b)

$\alpha_{ch,N} = (h_{ef} / 180)^{0.15} \leq 1.0$, (SI-units) (17.6.2.7.2b)

The modification factor to account for the influence of location and loading of adjacent anchors, $\psi_{s,N}$, shall be computed in accordance with Eq. (17.6.2.7.1a)

$$\psi_{s,N}=\frac{1}{1+\sum_{i=2}^{n+1}\left[\left(1-\frac{s_i}{s_{cr,N}}\right)^{1.5}\cdot\frac{N_{ua,i}^{a}}{N_{ua,1}^{a}}\right]} \qquad (17.6.2.7.1a)$$

where (as illustrated in Figure 8):

s_i = distance between the anchor under consideration and adjacent anchor, in. (mm)

$\leq s_{cr,N}$

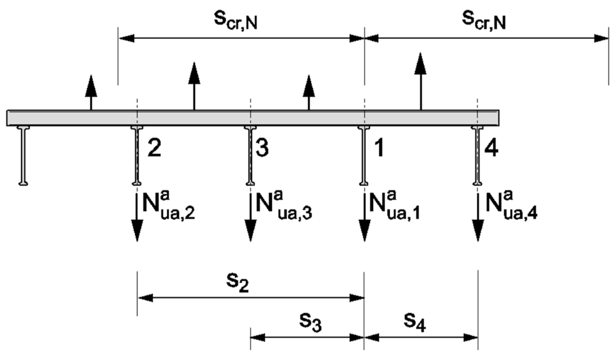
$s_{cr,N} = 2\,(2.8 - (1.3\,h_{ef} / 7.1))\,h_{ef} \geq 3\,h_{ef}$, in. (17.6.2.7.1b)

$s_{cr,N} = 2\,(2.8 - (1.3\,h_{ef} / 180))\,h_{ef} \geq 3\,h_{ef}$, mm (17.6.2.7.1b)

$N^{a}_{ua,i}$ = factored tension load of an influencing anchor, lbf (N)

$N^{a}_{ua,1}$ = factored tension load of the anchor under consideration, lbf (N)

n = number of anchors of all anchor channels within a radial distance $s_{cr,N}$ from the anchor under consideration



1 = anchor under consideration,

2 to 4 = influencing anchors

FIGURE 8—EXAMPLE OF ANCHOR CHANNEL WITH NON-UNIFORM ANCHOR TENSION FORCES

The modification factor for edge effect of anchors loaded in tension, $\psi_{ed,N}$, shall be computed in accordance with Eq. (17.6.2.7.5a) or (17.6.2.7.5b).

If $c_{a1} \geq c_{cr,N}$

then $\psi_{ed,N} = 1.0$ (17.6.2.7.5a)

If $c_{a1} < c_{cr,N}$

then $\psi_{ed,N} = (c_{a1} / c_{cr,N})^{0.5} \leq 1.0$ (17.6.2.7.5b)

where

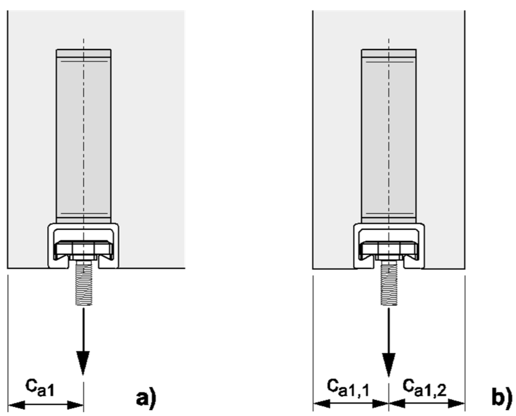
$c_{cr,N} = 0.5\ s_{cr,N}$

$= (2.8 - (1.3\ h_{ef} / 7.1))\ h_{ef} \geq 1.5\ h_{ef}$, in. (17.6.2.7.5c)

$c_{cr,N} = 0.5\ s_{cr,N}$

$= (2.8 - (1.3\ h_{ef} / 180))\ h_{ef} \geq 1.5\ h_{ef}$, mm (17.6.2.7.5c)

If anchor channels are located in a narrow concrete member with multiple edge distances $c_{a1,1}$ and $c_{a1,2}$ (as shown in Figure 9b), the minimum value of $c_{a1,1}$ and $c_{a1,2}$ shall be inserted in Eq. 17.6.2.7.5b).



at an edge in a narrow member

FIGURE 9—ANCHOR CHANNELS WITH EDGE(S)

The modification factor for corner effect for anchors loaded in tension (as illustrated in Figures 10a and 10b), $\psi_{co,N}$, shall be computed in accordance with Eq. (17.6.2.7.6a) or (17.6.2.7.6b)

If $c_{a2} \geq c_{cr,N}$

then $\psi_{co,N} = 1.0$ (17.6.2.7.6a)

If $c_{a2} < c_{cr,N}$

then $\psi_{co,N} = (c_{a2} / c_{cr,N})^{0.5} \leq 1.0$ (17.6.2.7.6b)

where

c_{a2} = distance of the anchor under consideration to the corner (see Figure 10 a, b, d)

If an anchor is influenced by two corners (as illustrated in Figure 10c), the factor $\psi_{co,N}$ shall be computed for each of the values $c_{a2,1}$ and $c_{a2,2}$ and the product of the factors, $\psi_{co,N}$, shall be inserted in Eq. (17.6.2.7.1).

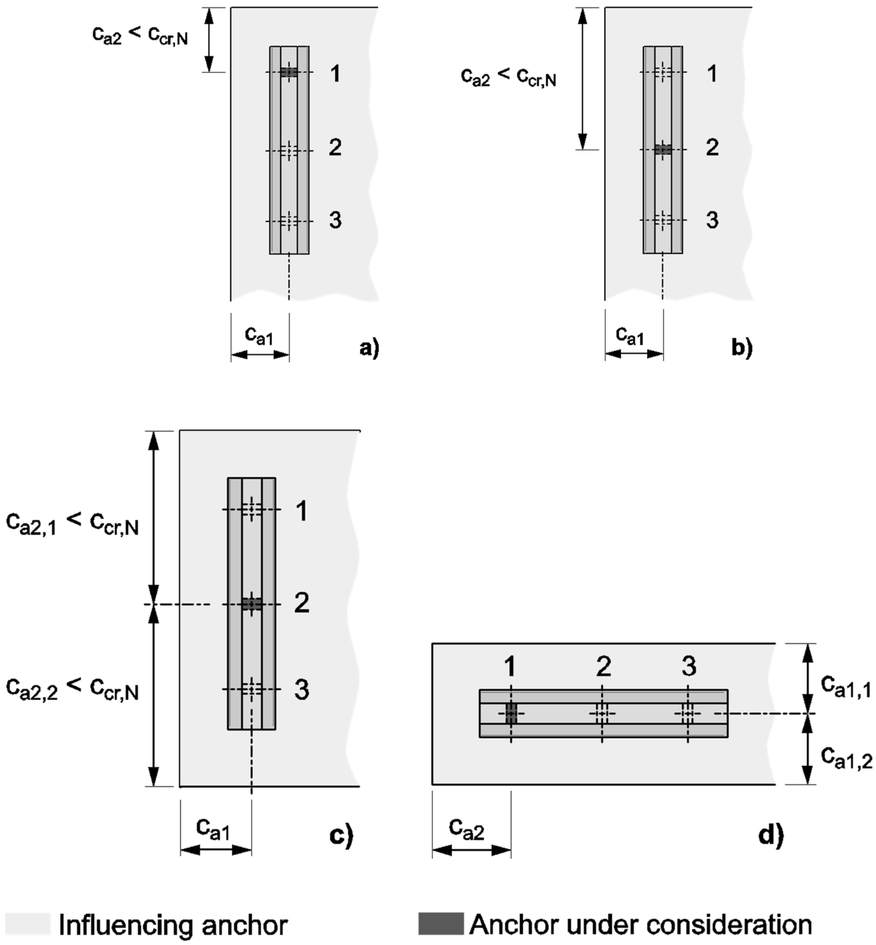


FIGURE 10—ANCHOR CHANNEL AT A CORNER OF A CONCRETE MEMBER

For anchor channels located in a region of a concrete member where analysis indicates no cracking at service load levels, the following modification factor shall be permitted:

$\psi_{c,N} = 1.25$.

Where analysis indicates cracking at service load levels, $\psi_{c,N}$, shall be taken as 1.0. The cracking in the concrete shall be controlled by flexural reinforcement distributed in accordance with ACI 318-19 Section 24.3.2 and 24.3.3, or equivalent crack control shall be provided by confining reinforcement.

The modification factor for anchor channels designed for uncracked concrete without supplementary reinforcement to control splitting, $\psi_{cp,N}$, shall be computed in accordance with Eq. (17.6.2.7.8a) or (17.6.2.7.8b). The critical edge distance, c_{ac}, shall be taken from Table 4 of this report.

If $c_{a,min} \geq c_{ac}$

then $\psi_{cp,N} = 1.0$ (17.6.2.7.8a)

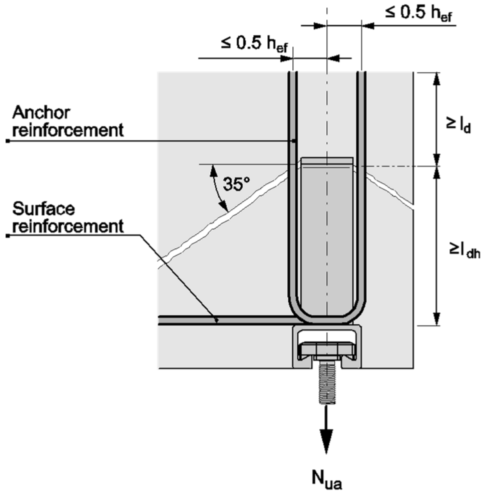
If $c_{a,min} < c_{ac}$

then $\psi_{cp,N} = c_{a,min} / c_{ac}$ (17.6.2.7.8b)

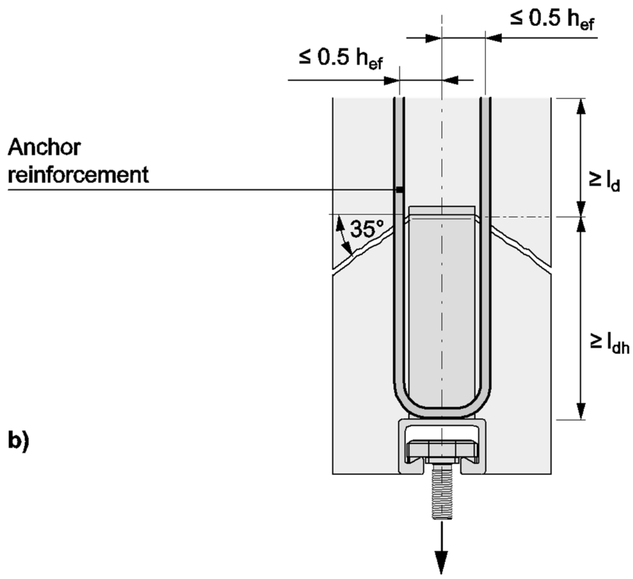
whereby $\psi_{cp,N}$ as determined in accordance with Eq.(17.6.2.7.8b) shall not be taken less than $c_{cr,N} / c_{ac}$ with $c_{cr,N}$ taken from Eq. (17.6.2.7.5c). For all other cases, $\psi_{cp,N}$ shall be taken as 1.0.

Where anchor reinforcement is developed in accordance with ACI 318-19 Chapter 25 on both sides of the breakout surface for an anchor of an anchor channel, the design strength of the anchor reinforcement, ϕN_{ca}, shall be permitted to be used instead of the concrete breakout strength, ϕN_{cb}, in determining ϕN_n. The anchor reinforcement for one anchor shall be designed for the tension force, N^a_{ua}, on this anchor using a strut-and-tie model. The provisions in Figure 11 shall be taken into account when sizing and detailing the anchor reinforcement. Anchor reinforcement shall consist of stirrups made from deformed reinforcing bars with a maximum diameter of $^5/_8$ in. (No. 5 bar) (16 mm). A strength reduction factor ϕ of 0.75 shall be used in the design of the anchor reinforcement.

For anchor channels located parallel to the edge of a concrete member or in a narrow concrete member, the plane of the anchor reinforcement shall be arranged perpendicular to the longitudinal axis of the channel (as shown in Figure 11 a, b).



a) Anchor channel parallel to an edge



b) Anchor channel in a narrow member

FIGURE 11—ARRANGEMENT OF ANCHOR REINFORCEMENT FOR ANCHOR CHANNELS LOADED BY TENSION LOAD

4.2.2.4 Pullout strength in tension: For anchors of anchor channels, the pullout strength N_{pn} shall be computed in accordance with Sections 17.6.3.1, 17.6.3.2, or 17.6.3.3 of ACI 318-19.

4.2.2.5 Concrete side-face blowout strength in tension: For anchor channels with deep embedment close to an edge (h_{ef} > 2.0 c_{a1}) the nominal side-face blowout strength, N_{sb}, of a single anchor shall be computed in accordance with Eq. (17.6.4.3.1).

$N_{sb} = N^0_{sb} \cdot \psi_{s,Nb} \cdot \psi_{g,Nb} \cdot \psi_{co,Nb} \cdot \psi_{h,Nb} \cdot \psi_{c,Nb}$, lbf (N) (17.6.4.3.1)

The basic nominal strength of a single anchor without influence of neighboring anchors, corner or member thickness effects in cracked concrete, N^0_{sb}, shall be computed in accordance with Eq. (17.6.4.3.2).

$N^0_{sb} = 128 \cdot \lambda \cdot c_{a1} \cdot \sqrt{A_{brg}} \cdot \sqrt{f_c'}$, lbf (17.6.4.3.2)

$N^0_{sb} = 10.5 \cdot \lambda \cdot c_{a1} \cdot \sqrt{A_{brg}} \cdot \sqrt{f_c'}$, N (17.6.4.3.2)

The modification factor accounting for the distance to and loading of neighboring anchors, $\psi_{s,Nb}$, shall be computed in accordance with Eq. (17.6.2.7.1a), however $s_{cr,N}$, shall be replaced by $s_{cr,Nb}$, which shall be computed in accordance with Eq. (17.6.4.3.3).

$s_{cr,Nb} = 4 \cdot c_{a1}$, in. (mm) (17.6.4.3.3)

The modification factor to account for influence of the bearing area of neighboring anchors, $\psi_{g,Nb}$, shall be computed in accordance with Eq. (17.6.4.3.4a) or Eq. (17.6.4.3.4b).

If $s \geq 4 \cdot c_{a1}$

then $\psi_{g,Nb} = 1.0$ (17.6.4.3.4a)

If $s < 4 \cdot c_{a1}$

then $$\psi_{g,Nb} = \sqrt{n} + \left(1 - \sqrt{n}\right) \cdot \frac{s}{4 \cdot c_{a1}} \geq 1.0 \quad (17.6.4.3.4b)$$

where:

n = number of tensioned anchors in a row parallel to the edge. For adjacent anchor channels, in Equation (17.6.4.3.4b), s shall be taken as the maximum of the individual anchor spacing s and the spacing between end anchors in the adjacent anchor channels $s_{ch,x}$.

The modification factor to account for influence of corner effects, $\psi_{co,Nb}$, shall be computed in accordance with *Eq*. (17.6.4.3.5a).

$$\psi_{co,Nb} = \left(\frac{c_{a2}}{c_{cr,Nb}}\right)^{0.5} \leq 1.0 \quad (17.6.4.3.5a)$$

where:

c_{a2} = corner distance of the anchor, for which the resistance is computed, in. (mm)

$c_{cr,Nb}$ = $2 \cdot c_{a1}$, in. (mm) (17.6.4.3.5b)

If an anchor is influenced by two corners ($c_{a2} < 2 \cdot c_{a1}$), then the factor $\psi_{co,Nb}$ shall be computed for $c_{a2,1}$ and $c_{a2,2}$ and the product of the factors shall be inserted in Eq. (17.6.4.3.1).

The modification factor to account for the influence of the member thickness, $\psi_{h,Nb}$, shall be computed in accordance with Eq. (17.6.4.3.6a) or Eq. (17.6.4.3.6b).

If $f > 2 \cdot c_{a1}$

then $\psi_{h,Nb} = 1.0$ (17.6.4.3.6a)

If $f \leq 2 \cdot c_{a1}$

then $$\psi_{h,Nb} = \frac{h_{ef} + f}{4 \cdot c_{a1}} \leq \frac{2 \cdot c_{a1} + f}{4 \cdot c_{a1}} \quad (17.6.4.3.6b)$$

where:

f = distance between the anchor head and the surface of the concrete member opposite to the anchor channel (as illustrated in Figure 12) in. (mm)

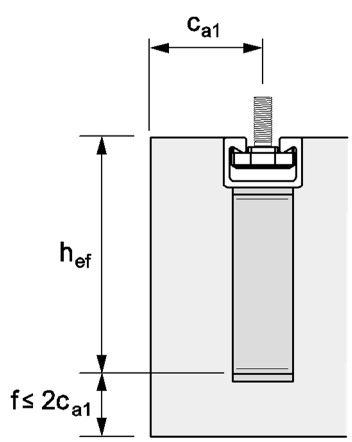


FIGURE 12—ANCHOR CHANNEL AT THE EDGE OF A THIN CONCRETE MEMBER

The modification factor to account for the influence of uncracked concrete, $\psi_{c,Nb}$, shall be in accordance with Section 4.2.2.3 of this report.

For anchor channels located perpendicular to the edge and loaded uniformly, verification is only required for the anchor closest to the edge.

4.2.3 Shear loads acting perpendicular to the channel axis:

4.2.3.1 General: Following verifications are required:

a) Steel failure: Strength of channel bolt, strength for local failure of channel lip, strength of connection between anchor and channel profile and strength of anchor, see Section 4.2.3.2

b) Concrete edge breakout strength of anchor channel in shear, see Section 4.2.3.3

c) Concrete pryout strength of anchor channel in shear, see Section 4.2.3.4

4.2.3.2 Steel strength of anchor channels in shear perpendicular to its longitudinal axis: For anchor channels, the nominal steel shear strength shall be determined as follows:

The nominal strength of a channel bolt in shear, V_{ss}, must be taken from Table 8 of this report.

If the fixture is not clamped against the concrete but secured to the channel bolt at a distance from the concrete surface (e.g. by double nuts), the nominal strength of a channel bolt in shear, $V_{ss,M}$, shall be computed in accordance with Eq. (17.7.1.3.1b).

$V_{ss,M} = (\alpha_M \cdot M_{ss}) / l$, lbf (N) (17.7.1.3.1b)

where

α_M = factor to take account of restraint of the fixture

= 1.0 if the fixture can rotate freely (no restraint)

= 2.0 if the fixture cannot rotate (full restraint)

$M_{ss} = M^0_{ss} \cdot \left(1 - \frac{N_{ua}}{\phi N_{ss}}\right)$, lbf-in. (Nm) (17.7.1.3.1c)

M^0_{ss} = nominal flexural strength of channel bolt. It shall be taken from Table 8 of this report

$\leq 0.5 \cdot N_{sl} \cdot a$

$\leq 0.5 \cdot N_{ss} \cdot a$

l = lever arm, in. (mm)

a = internal lever arm, in. (mm)

The nominal strength of the channel lips to take up shear loads perpendicular to the channel axis transmitted by a channel bolt, $V_{sl,y}$, shall be taken from Table 5 of this report.

The nominal strength of one anchor, $V_{sa,y}$, to take up shear loads perpendicular to the channel axis shall be taken from Table 5 of this report.

The nominal strength of the connection between one anchor and the anchor channel, $V_{sc,y}$, to take up shear loads perpendicular to the channel axis shall be taken from Table 5 of this report.

4.2.3.3 Concrete breakout strength of an anchor channel in shear perpendicular to its longitudinal axis: The nominal concrete breakout strength, $V_{cb,y}$, in shear perpendicular to the channel axis of a single anchor of an anchor channel in cracked concrete shall be computed as follows:

a) For a shear force perpendicular to the edge, by Eq. (17.7.2.7.1)

$V_{cb,y} = V_b \cdot \psi_{s,V} \cdot \psi_{co,V} \cdot \psi_{c,V} \cdot \psi_{h,V}$, lbf (N) (17.7.2.7.1)

b) For a shear force parallel to an edge (as shown in Figure 13), $V_{cb,y}$, shall be permitted to be 2.5 times the value of the shear force determined from Eq. (D-21.a) with the shear force assumed to act perpendicular to the edge.

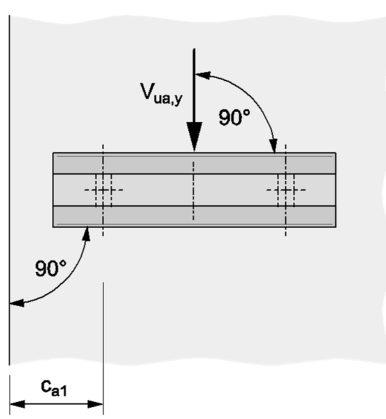


FIGURE 13—ANCHOR CHANNEL ARRANGED PERPENDICULAR TO THE EDGE AND LOADED PARALLEL TO THE EDGE

The basic concrete breakout strength in shear perpendicular to the channel axis of a single anchor of an anchor channel in cracked concrete, V_b, shall be computed in accordance with Eq. (17.7.2.7.2).

$V_b = \lambda \cdot \alpha_{ch,V} \cdot (f'_c)^{0.5} \cdot c_{a1}^{4/3}$, lbf (N) (17.7.2.7.2)

where

λ = 1 (normal-weight concrete)

$\alpha_{ch,V}$ = shall be taken from Table 6 of this report

f'_c = the lesser of the specified concrete compressive strength and 8,500 psi (58.6 mPa)

The modification factor to account for the influence of location and loading of adjacent anchors, $\psi_{s,V}$, shall be computed in accordance with Eq. (17.7.2.7.3a).

$$\psi_{s,V} = \frac{1}{1 + \sum_{i=2}^{n+1} \left[\left(1 - \frac{s_i}{s_{cr,V}} \right)^{1.5} \cdot \frac{V^a_{ua,y,i}}{V^a_{ua,y,1}} \right]} \qquad (17.7.2.7.3a)$$

where (as illustrated in Figure 14):

s_i = distance between the anchor under consideration and the adjacent anchor, in. (mm)

$\leq$ $s_{cr,V}$

$s_{cr,V}$ = $4c_{a1} + 2b_{ch}$, in. (mm) (17.7.2.7.3b)

$V^a_{ua,y,i}$ = factored shear load of an influencing anchor, lbf (N),

$V^a_{ua,y,1}$ = factored shear load of the anchor under consideration, lbf (N),

n = number of anchors of all anchor channels within a radial distance $s_{cr,V}$ from the anchor under consideration

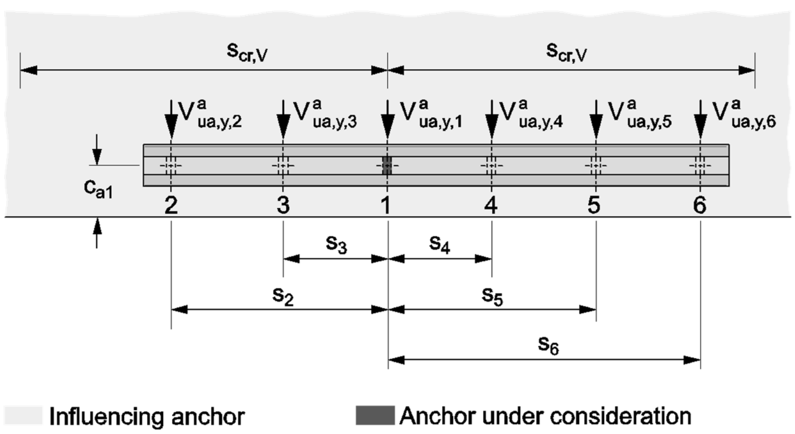


FIGURE 14—EXAMPLE OF AN ANCHOR CHANNEL WITH DIFFERENT ANCHOR SHEAR FORCES

The modification factor for corner effect for an anchor loaded in shear perpendicular to the channel axis (as shown in Figure 15a), $\psi_{co,V}$, shall be computed in accordance with Eq. (17.7.2.7.3c) or (17.7.2.7.3d).

If $c_{a2} \geq c_{cr,V}$

then $\psi_{co,V} = 1.0$ (17.7.2.7.3c)

If $c_{a2} < c_{cr,V}$

then $\psi_{co,V} = (c_{a2} / c_{cr,V})^{0.5}$ (17.7.2.7.3d)

where

$c_{cr,V} = 2c_{a1} + b_{ch}$, in. (mm) (17.7.2.7.3e)

If an anchor is influenced by two corners (as shown in Figure 15b), then the factor $\psi_{co,V}$ shall be computed for each corner in accordance with Eq. (17.7.2.7.3c) or (17.7.2.7.3d) and the product of the values of $\psi_{co,V}$ shall be inserted in Eq. (17.7.2.7.1).

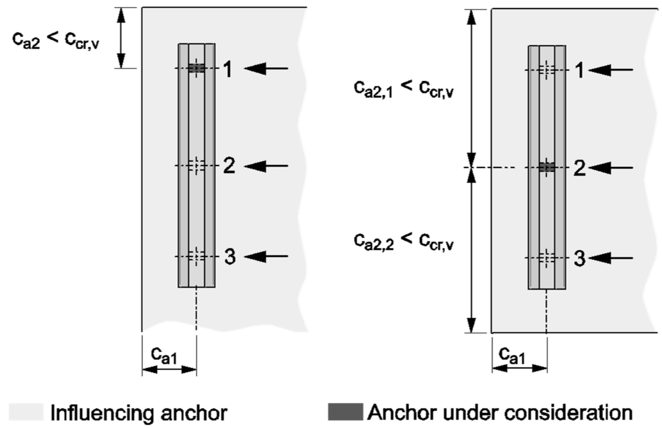


FIGURE 15—EXAMPLE OF AN ANCHOR CHANNEL LOADED IN SHEAR WITH ANCHORS:

a) influenced by one corner

b) influenced by two corners

For anchor channels located in a region of a concrete member where analysis indicates no cracking at service load levels, the following modification factor shall be permitted

$\psi_{c,V}$ = 1.4.

For anchor channels located in a region of a concrete member where analysis indicates cracking at service load levels, the following modifications shall be permitted:

$\psi_{c,V}$ = 1.0 for anchor channels in cracked concrete with no supplementary reinforcement

$\psi_{c,V}$ = 1.2 for anchor channels in cracked concrete with edge reinforcement of a No. 4 bar (12.7 mm) or greater between the anchor channel and the edge in accordance with Figure 16.

$\psi_{c,V}$ = 1.4 for anchor channels in cracked concrete containing edge reinforcement with a diameter of $^1/_2$ inch (12.7 mm) or greater (No. 4 bar or greater) between the anchor channel and the edge, and with the edge reinforcement enclosed within stirrups with a diameter of $^1/_2$ inch (12.7 mm) or greater (No. 4 bar or greater) spaced at 8 inches (200 mm) maximum.

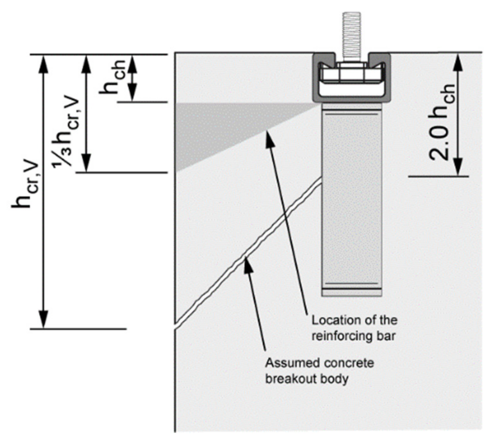


FIGURE 16—RECOMMENDED AREA FOR THE LOCATION OF THE EDGE REINFORCEMENT BAR (Reinforcing bar location within recommended area shall account for all factors, (for example, concrete cover, bend radius, etc.) as required by ACI 318

The modification factor for anchor channels located in a concrete member with $h < h_{cr,V}$, $\psi_{h,V}$ (an example is given in Figure 17), shall be computed in accordance with Eq. (17.7.2.7.6a).

$\psi_{h,V} = (h / h_{cr,V})^{1/2} \leq 1.0$ (17.7.2.7.6a)

where

$h_{cr,V} = 2c_{a1} + 2h_{ch}$, in. (mm) (17.7.2.7.6b)

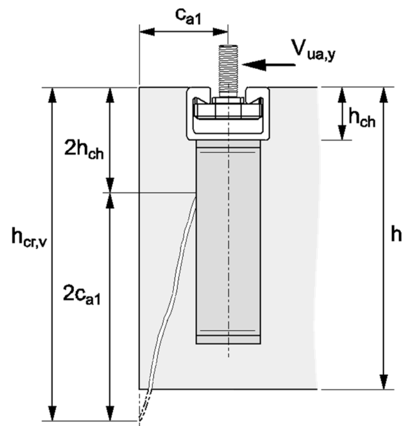


FIGURE 17—EXAMPLE OF AN ANCHOR CHANNEL IN A MEMBER WITH A THICKNESS $h < h_{cr,V}$

Where an anchor channel is located in a narrow member ($c_{a2,max} < c_{cr,V}$) with a thickness $h < h_{cr,V}$ (see Figure 18), the edge distance c_{a1} in Eq. (17.7.2.7.2), (17.7.2.7.3b), (17.7.2.7.3e) and (17.7.2.7.6b) shall not exceed the value $c_{a1,red}$ determined in accordance with Eq. (17.7.2.7.7).

$$c_{a1,red} = \max\left[\frac{c_{a,max} - b_{ch}}{2}; \frac{h - 2h_{ch}}{2}\right], \text{ in. (mm)} \quad (17.7.2.7.7)$$

where $c_{a2,max}$ is the largest of the edge distances perpendicular to the longitudinal axis of the channel.

For this example, the value of $c_{a1,red}$ is obtained by moving the failure surface forward until it intersects the corner as shown.

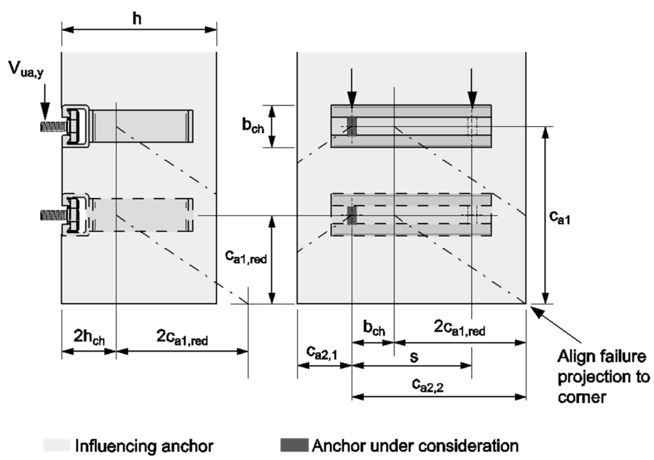


FIGURE 18—EXAMPLE OF AN ANCHOR CHANNEL INFLUENCED BY TWO CORNERS AND MEMBER THICKNESS (IN THIS EXAMPLE $c_{a2,2}$ IS DECISIVE FOR THE DETERMINATION OF $c_{a1,red}$)

For anchor channels with b_{ch} greater than 1.1 in. (28 mm) and h_{ch} greater than 0.6 in. (15 mm) arranged parallel to the edge and loaded by a shear load perpendicular to the edge and anchor reinforcement developed in accordance with ACI 318-19 Chapter 25 on both sides of the concrete surface, the design strength of the anchor reinforcement, $\phi V_{ca,y}$, shall be permitted to be used instead of the concrete breakout strength, $\phi V_{cb,y}$, in determining $\phi V_{n,y}$.

A strength reduction factor ϕ of 0.75 shall be used in the design of the anchor reinforcement. The strength of the anchor reinforcement assumed in design shall not exceed the value in accordance with Eq. (17.7.2.7.8.1). Only anchor reinforcement that complies with Figure 19 shall be assumed as effective.

The maximum strength of the anchor reinforcement $V_{ca,y,max}$ of a single anchor of an anchor channel shall be computed in accordance with Eq. (17.7.2.7.8.1).

$V_{ca,y,max}$ = 2.85 / (ca1)0.12 · $V_{cb,y}$, lbf (17.7.2.7.8.1)

$V_{ca,y,max} = 4.20 / (c_{a1})^{0.12} \cdot V_{cb,y}$, N (17.7.2.7.8.1)

where $V_{cb,y}$ is determined in accordance with Eq. (17.7.2.7.1).

Anchor reinforcement shall consist of stirrups made from deformed reinforcing steel bars with a maximum diameter of 5/8 in. (16 mm) (No. 5 bar) and straight edge reinforcement with a diameter not smaller than the diameter of the stirrups (as shown in Figure 19). Only one bar at both sides of each anchor shall be assumed as effective. The distance of this bar from the anchor shall not exceed $0.5c_{a1}$ and the anchorage length in the breakout body shall be not less than 4 times the bar diameter. The distance between stirrups shall not exceed the smaller of anchor spacing or 6 in. (152 mm).

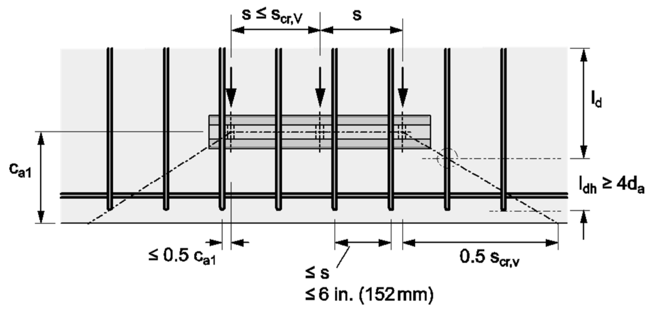


FIGURE 19—REQUIREMENTS FOR DETAILING OF ANCHOR REINFORCEMENT OF ANCHOR CHANNELS

The anchor reinforcement of an anchor channel shall be designed for the highest anchor load, $V^a_{ua,y}$, of all anchors but at least for the highest individual shear load, $V^b_{ua,y}$, acting on the channel. This anchor reinforcement shall be arranged at all anchors of an anchor channel.

For anchor channels in a parallel configuration, it shall be permitted to calculate the concrete breakout strength either for the anchor channel closest to the edge or the anchor channel furthest from the edge. The nominal concrete breakout strength shall be computed as follows:

a) For verification of the anchor channel closest to the edge, the nominal concrete breakout strength shall be calculated according to Eq. (17.7.2.9a).

$V_{cb} = min\,(n_{ch} \cdot V_{cb}(c_{a1,1});\ V_{cb}(c_{a1,n_1}))$, lbf (17.7.2.9a)

b) For verification of the anchor channel furthest from the edge, the nominal concrete breakout strength shall be calculated according to Eq. (D-29.f).

$V_{cb} = V_{cb}(c_{a1,n_1})$, lbf (17.7.2.9b)

For case b, the anchor channels closer to the edge shall be assumed to carry zero tension and shear load.

4.2.3.4 Concrete pryout strength of anchor channels in shear perpendicular to the channel axis: The nominal pryout strength, V_{cp}, in shear of a single anchor of an anchor channel without anchor reinforcement shall be computed in accordance with Eq. (17.7.3.2).

$V_{cp} = V_{cp,y} = k_{cp}\, N_{cb}$, lbf (N) (17.7.3.2)

where

k_{cp} = factor taken from Table 6 of this report

N_{cb} = nominal concrete breakout strength of the anchor under consideration, lbf (N), determined in accordance with 4.2.2.3; however in the determination of the modification factor $\psi_{s,N}$, the values $N^a_{ua,1}$ and $N^a_{ua,i}$ in Eq. (D-9.a) shall be replaced by $V^a_{ua,y,1}$ and $V^a_{ua,y,i}$, respectively.

The nominal pryout strength, V_{cp}, in shear of a single anchor of an anchor channel with anchor reinforcement shall not exceed.

$V_{cp} = V_{cp,y} = 0.75 \cdot k_{cp} \cdot N_{cb}$, lbf (N) (17.7.3.3)

where k_{cp} and N_{cb} as defined above.

4.2.4 Shear loads acting longitudinal to the channel axis:

4.2.4.1 General: Following verifications are required:

a) Steel failure: Strength of channel bolt, strength for local failure of channel lip, strength of connection between anchor and channel profile and strength of anchor, see Section 4.2.4.2.

b) Concrete edge breakout strength of anchor channel in shear, see Section 4.2.4.3.

c) Concrete pryout strength of anchor channel in shear, see Section 4.2.4.4.

4.2.4.2 Steel strength of anchor channels in shear: For anchor channels, the nominal steel shear strength shall be determined as follows:

The nominal strength of a channel bolt in shear, V_{ss}, shall be taken from Table 8 of this report.

If the fixture is not clamped against the concrete but secured to the channel bolt at a distance from the concrete surface (e.g. by double nuts), the nominal strength of a channel bolt in shear, $V_{ss,M}$, shall be computed in accordance with Eq. (17.7.1.3.1b).

The nominal strength of the channel lips to take up shear loads in direction of the longitudinal channel axis transmitted by a channel bolt, $V_{sl,x}$, shall be taken from Table 5 of this report.

The nominal strength of one anchor, $V_{sa,x}$, to take up shear loads longitudinal to the channel axis shall be taken from Table 5 of this report.

The nominal strength of the connection between one anchor and the anchor channel, $V_{sc,x}$, to take up shear loads longitudinal to the channel axis shall be taken from Table 5 of this report.

4.2.4.3 Concrete breakout strength of an anchor channel in shear: The nominal concrete breakout strength, $V_{cb,x}$, in shear in direction of the longitudinal channel axis of a single anchor of an anchor channel in cracked concrete shall be computed as follows:

a) For a shear force perpendicular to the edge, by Eq. (17.7.2.7.1). The basic concrete breakout strength in shear in direction of the longitudinal channel axis of a single anchor of an anchor channel in cracked concrete, V_b, shall be computed in accordance with Eq. (17.7.2.7.2).

b) For a shear force parallel to an edge, $V_{cb,x}$, shall be permitted to be 2 times the value of the shear force determined from Eq. (17.7.2.7.1) with the shear force assumed to act perpendicular to the edge.

4.2.4.4 Concrete pryout strength in shear: The nominal pryout strength, $V_{cp,x}$, in shear of a single anchor of an anchor channel without anchor reinforcement shall be computed in accordance with Eq. (17.7.3.2).

The nominal pryout strength, $V_{cp,x}$, in shear of a single anchor of an anchor channel with anchor reinforcement shall not exceed Eq. (17.7.3.3).

4.2.5 Requirements for seismic design: Anchor channels shall be designed according to ACI 318-19 17.10.6.3.

The design of channels to resist tension loads in SDC C, D, E or F where ACI 318-19 17.10.5.2 applies shall satisfy the requirements of ACI 318-19 17.10.5.3 (b), (c), or (d). The design of anchor channels to resist shear loads in SDC C, D, E or F where ACI 318-19 17.10.6.2 applies shall satisfy the requirements of ACI 318-19 17.10.6.3.

For anchor channels in SDC C, D, E or F the design strengths given in Section 4.2.1 through Section 4.2.4 shall be taken as the corresponding seismic strengths $\phi N_{n,seis}$, $\phi V_{n,y,seis}$ and $\phi V_{n,x,seis}$.

4.2.6 Interaction of tensile and shear forces: For designs that include combined tensile and shear forces, the interaction of these loads has to be verified.

Anchor channels subjected to combined axial and shear loads shall be designed to satisfy the following requirements by distinguishing between steel failure of the channel bolt, steel failure modes of the anchor channel and concrete failure modes.

4.2.6.1 Steel failure of channel bolts under combined loads: For channel bolts, Eq. (17.8.4.1) shall be satisfied

$$\left(\frac{N_{ua}^{b}}{\phi N_{ss}}\right)^{2}+\left(\frac{V_{ua}^{b}}{\phi V_{ss}}\right)^{2}\leq 1.0 \qquad (17.8.4.1)$$

with $V_{ua}^{b}=\sqrt{\left(V_{ua,x}^{b}\right)^{2}+\left(V_{ua,y}^{b}\right)^{2}}$

where N^{b}_{ua} is the factored tension load, $V^{b}_{ua,y}$ is the factored shear load in perpendicular direction, and $V^{b}_{ua,x}$ is the factored shear load in longitudinal direction to the channel axis on the channel bolt under consideration.

This verification is not required in case of shear load with lever arm as Eq. (17.7.1.3.1b) accounts for the interaction.

4.2.6.2 Steel failure modes of anchor channels under combined loads: For steel failure modes of anchor channels Eq. (17.8.4.2.1), (17.8.4.2.2a) and (17.8.4.2.2b) shall be satisfied.

a) For anchor and connection between anchor and channel profile:

$$\max\left(\frac{N_{ua}^{a}}{\phi N_{sa}};\frac{N_{ua}^{a}}{\phi N_{sc}}\right)^{\alpha}+\max\left(\frac{V_{ua,y}^{a}}{\phi V_{sa,y}};\frac{V_{ua,y}^{a}}{\phi V_{sc,y}}\right)^{\alpha}+\max\left(\frac{V_{ua,x}^{a}}{\phi V_{sa,x}};\frac{V_{ua,x}^{a}}{\phi V_{sc,x}}\right)^{2}\leq 1.0 \qquad (17.8.4.2.1)$$

where $\alpha = 2$ for anchor channels with max ($V_{sa,y}; V_{sc,y}$) ≤ min ($N_{sa}; N_{sc}$)

$\alpha = 1$ for anchor channels with max ($V_{sa,y}; V_{sc,y}$) > min ($N_{sa}; N_{sc}$)

b) At the point of load application:

$$\left(\frac{N_{ua}^{b}}{\phi N_{sl}}\right)^{\alpha}+\left(\frac{V_{ua,y}^{b}}{\phi V_{sl,y}}\right)^{\alpha}+\left(\frac{V_{ua,x}^{b}}{\phi V_{sl,x}}\right)^{2}\leq 1.0 \qquad (17.8.4.2.2a)$$

$$\left(\frac{M_{u,flex}}{\phi M_{s,flex}}\right)^{\alpha}+\left(\frac{V_{ua,y}^{b}}{\phi V_{sl,y}}\right)^{\alpha}+\left(\frac{V_{ua,x}^{b}}{\phi V_{sl,x}}\right)^{2}\leq 1.0 \qquad (17.8.4.2.2b)$$

where $\alpha = 2$ for anchor channels with $V_{sl,y} \leq N_{s,l}$

$\alpha = 1$ for anchor channels with $V_{sl,y} > N_{s,l}$

4.2.6.3 Concrete failure modes of anchor channels under combined loads: For concrete failure modes, anchor channels shall be designed to satisfy the requirements in a) through d).

a) If $\left(\frac{V_{ua,y}^{a}}{\phi V_{nc,y}}\right)+\left(\frac{V_{ua,x}^{a}}{\phi V_{nc,x}}\right)\leq 0.2$ then the full strength in tension shall be permitted: $\phi N_{nc} \geq N_{ua}^{a}$

b) If $N_{ua}^{a} \leq 0.2\phi N_{nc}$ then the full strength in shear shall be permitted: $\left(\frac{V_{ua,y}^{a}}{\phi V_{nc,y}}\right)+\left(\frac{V_{ua,x}^{a}}{\phi V_{nc,x}}\right)\leq 1.0$

c) If $\left(\frac{V_{ua,y}^{a}}{\phi V_{nc,y}}\right)+\left(\frac{V_{ua,x}^{a}}{\phi V_{nc,x}}\right)> 0.2$ and $N_{ua}^{a} > 0.2\phi N_{nc}$ then Eq. (17.8.4.3.3a) applies.

$$\left(\frac{N_{ua}^{a}}{\phi N_{nc}}\right)+\left(\frac{V_{ua,y}^{a}}{\phi V_{nc,y}}\right)+\left(\frac{V_{ua,x}^{a}}{\phi V_{nc,x}}\right)\leq 1.2 \qquad (17.8.4.3.3a)$$

d) Alternatively, instead of satisfying the requirements in a) through c), the interaction Eq. (17.8.4.3.3b) shall be satisfied:

$$\left(\frac{N_{ua}^{a}}{\phi N_{nc}}\right)^{5/3}+\left(\frac{V_{ua,y}^{a}}{\phi V_{nc,y}}\right)^{5/3}+\left(\frac{V_{ua,x}^{a}}{\phi V_{nc,x}}\right)^{5/3}\leq 1.0 \qquad (17.8.4.3.3b)$$

Where anchors consist of deformed reinforcing bars in accordance with Section 3.1, and the deformed reinforcing bars are lap spliced with reinforcing bars in the member according to the requirements of ACI 318-19 Section 25.5 the interaction equation (17.8.4.3.3c) shall be satisfied.

$$\left(\frac{V_{ua,y}^{a}}{\phi V_{nc,y}}\right)^{5/3}+\left(\frac{V_{ua,x}^{a}}{\phi V_{nc,x}}\right)^{5/3}\leq \alpha \qquad (17.8.4.3.3c)$$

where

α = 0.9 for anchor channels with deformed reinforcing bars not de-bonded

α = 1.0 for anchor channels with deformed reinforcing bars de-bonded underneath the channel profile for a length of 2 in. (50mm).

4.2.7 Minimum member thickness, anchor spacing, and edge distance: Anchor channels shall satisfy the requirements for edge distance, anchor spacing, and member thickness.

The minimum edge distance, minimum and maximum anchor spacing, and minimum member thickness shall be taken from Table 1 of this report.

The critical edge distance, c_{ac}, shall be taken from Table 4 of this report.

4.3 Allowable stress design:

4.3.1 General: Strength design values determined in accordance with ACI 318-19 Chapter 17, as applicable, with amendments in Section 4.2 of this report may be converted to values suitable for use with allowable stress design (ASD) load combinations. Such guidance of conversions shall be in accordance with the following:

For anchor channels designed using load combinations in accordance with IBC Section 1605.1 (Allowable Stress Design), allowable loads shall be established using Eq.(3.1), Eq.(3.2), Eq.(3.3), or Eq.(3.4):

$T_{allowable,ASD} = \phi N_n / \alpha_{ASD}$ Eq.(3.1)

$V_{x,allowable,ASD} = \phi V_{n,x} / \alpha_{ASD}$ Eq.(3.2)

$V_{y,allowable,ASD} = \phi V_{n,y} / \alpha_{ASD}$ Eq.(3.3)

$M_{s,flex,allowable,ASD} = \phi M_{s,flex} / \alpha_{ASD}$ Eq.(3.4) where:

$T_{allowable,ASD}$ = allowable tension load, lbf (N)

$V_{x,allowable,ASD}$ = allowable shear load longitudinal to the channel axis, lbf (N)

$V_{y,allowable,ASD}$ = allowable shear load perpendicular to the channel axis, lbf (N)

$M_{s,flex,allowable,ASD}$ = allowable bending moment due to tension loads lbf-in. (Nm)

ϕN_n = lowest design strength of an anchor, channel bolt, or anchor channel in tension for controlling failure mode as determined in accordance with ACI 318-19 Chapter 17, as applicable, with amendments in Section 4.2 of this report, lbf (N).

$\phi V_{n,x}$ = lowest design strength of an anchor, channel bolt, or anchor channel in shear longitudinal to the channel axis for controlling failure mode as determined in accordance with ACI 318-19 Chapter 17 as applicable with amendments in Section 4.2 of this report, lbf (N).

$\phi V_{n,y}$ = lowest design strength of an anchor, channel bolt, or anchor channel in shear perpendicular to the channel axis for controlling failure mode as determined in accordance with ACI 318-19 Chapter 17, as applicable, with amendments in Section 4.2 of this report, lbf (N).

α_{ASD} = conversion factor calculated as a weighted average of the load factors for the controlling load combination. In addition, α_{ASD} shall include all applicable factors to account for non-ductile failure modes and required overstrength.

4.3.2 Interaction of tensile and shear forces: Interaction shall be calculated in accordance with Section 4.2.4 and amendments in Section 4.2 of this report.

N_{ua}, $V_{ua,x}$, $V_{ua,y}$ and $M_{u,flex}$ shall be replaced by the unfactored loads T^a, $V^a{}_x$, $V^a{}_y$, M^a. The design strengths ϕN_n, $\phi V_{n,x}$, $\phi V_{n,y}$ and $M_{s,flex}$ shall be replaced by the allowable loads $T_{allowable,ASD}$, $V_{x,allowable,ASD}$, $V_{y,allowable,ASD}$ and $M_{s,flex,allowable,ASD}$.

where

T^a = unfactored tension load applied to an anchor channel, lbf (N)

M^a = unfactored bending moment on anchor channel due to tension loads, lbf-in. (Nm)

$V^a{}_x$ = unfactored shear load applied to an anchor channel longitudinal to the channel axis, lbf (N)

$V^a{}_y$ = unfactored shear load applied to an anchor channel perpendicular to the channel axis, lbf (N)

4.4 Installation:

Installation parameters are provided in Table 1 of this report. Anchor channel locations shall comply with this report and the plans and specifications approved by the building official. Installation of the anchor channels and channel bolts shall conform to the manufacturer's printed installation instructions (MPII) included in each shipment, as provided in Tables 10 through 12 of this report.

Channel installation in formwork includes the following steps according to Table 10 of this report:

1. Selection of anchor channel, in accordance to the construction document;
2. Placing channel into formwork. Anchor channel must be flush with the concrete surface;
   a. Steel formwork: Fixing with HALFEN channel bolts through formwork penetration;
   b. Steel formwork: Fixing with rivets;
   c. Steel formwork: Fixing with HALFEN fixing cone;
   d. Timber formwork: Fixing with nails;
   e. Timber formwork: Fixing with staples;
   f. Fixing in the top surface of concrete: Fixing by using auxiliary construction;
   g. Fixing in the top surface of concrete: Fixing from above directly to the reinforcement; and
   h. Fixing in the top surface of concrete: Fixing to the reinforcement, using the HALFEN ChanClip.
3. Cast in and compact the concrete;
4. Hardening of the concrete;
5. Striking the formwork; and
6. Removing the combi strip filler.

Channel bolt installation in the anchor channel shall include the following steps according to Table 11 of this report:

1. Selection of the HALFEN channel bolts in accordance with the planning document.

2. Insert the channel bolt into the channel. After a 90° turn clockwise, the channel bolt locks into the channel. (Check of the position of the bolt by notch).
3. Positioning of the channel bolt: At the channel ends a minimum clearance must be maintained, which corresponds with the overhang beyond the last anchor.
4. Tighten the hexagonal nut to the setting torque (T_{inst}) acc. Table 12. T_{inst} must not be exceeded.
   4.1: general.
   4.2 and 4.3: steel-to-steel contact.
5. After fixing the nuts, check the correct position of the bolt: If the notch is not perpendicular to the channel length axis, the channel bolt must be released completely, inserted and tightened again.

4.5 Special inspection:

Periodic special inspection shall be performed as required in accordance with Section 1705.1.1 and Table 1705.3 of the IBC and in accordance with this report. For each type of anchor channel, the manufacturer shall provide inspection procedures to verify proper usage.

4.5.1 Inspection requirements: Prior to concrete placement, the special inspector shall inspect the placement of anchor channels in the formwork to verify anchor channel type, channel size, anchor type, number of anchors, anchor size, and length of anchors, as well as anchor channel location, position, orientation, and edge distance in accordance with the construction documents. The special inspector shall also verify that anchor channels are secured within the formwork in accordance with the manufacturer's printed installation instructions (MPII).

Following placement of concrete and form removal, the special inspector shall verify that the concrete around the anchor channel is without significant visual defects, that the anchor channel is flush with the concrete surface, and that the channel interior is free of concrete, laitance, or other obstructions. When anchor channels are not flush with the concrete surface, the special inspector shall verify that appropriate sized shims are provided in accordance with the MPII. Following the installation of attachments to the anchor channel, the special inspector shall verify that the specified system hardware, such as T-headed channel bolts and washers, have been used and positioned correctly, and the installation torque has been applied to the channel bolts in accordance with the installation instructions (MPII). The special inspector shall confirm with the engineer of record that the attachments do not produce gravity, wind, and/or seismic loading parallel to the longitudinal axis of the channel.

The special inspector shall be present for the installations of attachments to each type and size of anchor channel.

Where they exceed the requirements stated here, the special inspector shall adhere to the special inspection requirements provided in the statement of special inspections as prepared by the registered design professional in responsible charge.

4.5.2 Proof loading program: Where required by the registered design professional in responsible charge, a program for on-site proof loading (proof loading program) to be conducted as part of the special inspection shall include at a minimum the following information:

1. Frequency and location of proof loading based on channel size and length;
2. Proof loads specified by channel size and channel bolt;
3. Acceptable displacements at proof load;
4. Remedial action in the event of failure to achieve proof load or excessive displacement.

5.0 CONDITIONS OF USE:

The HALFEN HTA anchor channel and HS/HZS/HSR channel bolts described in this report are a suitable alternative to what is specified in those codes listed in Section 1.0 of this report, subject to the following conditions:

5.1 The HTA anchor channels and HS/HZS channel bolts are evaluated for use to resist static short- and long-term loads, including wind and seismic loads (IBC seismic design categories A and B) tension loads (N_{ua}) and shear loads perpendicular to the longitudinal channel axis ($V_{ua,y}$), or any combination of these loads applied at any location between the outermost anchors of the anchor channel in accordance with Figure 1 of this report.

The HTA anchor channel (HTA 40/22) and HALFEN HSR channel bolts are evaluated for use to resist static short- and long-term loads, including wind and seismic loads (IBC Seismic Design Categories A through F) tension loads (N_{ua}) and shear loads perpendicular to the longitudinal channel axis ($V_{ua,y}$), shear loads acting in the direction of the longitudinal channel axis ($V_{ua,x}$) or any combination of these loads applied at any location between the outermost anchors of the anchor channel in accordance with Figure 1 of this report.

5.2 The anchor channels and channel bolts shall be installed in accordance with Table 1 and the manufacturer's printed installation instructions (MPII), as included in the shipment and as shown in Table 10 through 12 of this report. In case of a conflict, this report governs.

5.3 The anchor channels shall be installed in cracked or uncracked normal-weight concrete having a specified compressive strength f'_c = 2,500 psi to 10,000 psi (17.2 MPa to 69.0 MPa) [minimum of 24 MPa is required under ADIBC Appendix L, Section 5.1.1].

5.4 The use of these anchor channels in lightweight concrete is beyond the scope of this report.

5.5 Strength design values shall be established in accordance with Section 4.2 of this report.

5.6 Allowable stress design values are established with Section 4.3 of this report.

5.7 Minimum and maximum anchor spacing and minimum edge distance as well as minimum member thickness shall comply with the values given in this report.

5.8 Prior to anchor channel installation, calculations and details demonstrating compliance with this report shall be submitted to the code official. The calculations and details shall be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is to be constructed.

5.9 Where not otherwise prohibited by the code, HALFEN HTA anchor channels are permitted for use with fire-resistance-rated construction provided that at least one of the following conditions is fulfilled:

- Anchor channels are used to resist wind or seismic forces only.
- Anchor channels that support a fire-resistance-rated envelope or a fire-resistance-rated membrane are protected by approved fire-resistance-rated materials, or have been evaluated for resistance to fire exposure in accordance with recognized standards.
- Anchor channels are used to support nonstructural elements.

5.10 Since an acceptance criteria for evaluating data to determine the performance of anchor channels subjected to fatigue or shock loading is unavailable at this time, the use of these anchor channels under such conditions is beyond the scope of this report.

5.11 Use of hot-dip galvanized carbon steel and stainless steel anchor channels is permitted for exterior exposure or damp environments.

5.12 Steel anchoring materials in contact with preservative-treated and fire-retardant-treated wood shall be of zinc-coated carbon steel or stainless steel. The minimum coating weights for zinc-coated steel shall comply with ASTM A153.

5.13 Special inspection shall be provided in accordance with Section 4.5 of this report.

5.14 HALFEN anchor channels and channel bolts are produced under an approved quality-control program with regular inspections performed by ICC-ES.

6.0 EVIDENCE SUBMITTED

6.1 Data in accordance with ICC-ES Acceptance Criteria for Anchor Channels in Concrete Elements AC232 (24), published April 2025.

6.2 Quality-control documentation.

7.0 IDENTIFICATION

7.1 The ICC-ES mark of conformity, electronic labeling, or the evaluation report number (ICC-ES ESR-1008) along with the name, registered trademark, or registered logo of the report holder must be included in the product label.

7.2 The anchor channels are identified by the manufacturer's name, anchor channel type and size (e.g. HTA 52/34) embossed into the channel profile or printed on the channel profile. For anchor channels made of stainless steel the anchor channel description will be followed by "A4" indicating the stainless-steel grade. The marking is visible after installation of the anchor channel.

7.3 The channel bolts are identified by packaging labeled with the manufacturer's name, bolt type, bolt diameter and length, bolt grade, and corrosion protection type (e.g. HS 38/17 M12 x 50 A4-70).

7.4 The report holders contact information is the following:

LEVIAT GMBH
LIEBIGSTRASSE 14
40764 LANGENFELD-RICHRATH
GERMANY
www.leviat.com

8.0 NOTATIONS

Equations are provided in units of inches and pounds. For convenience, SI (metric) units are provided in parentheses where appropriate. Unless otherwise noted, values in SI units shall be not used in equations without conversion to units of inches and pounds.

b_{ch} width of channel, as shown in Figure 20, in. (mm)

c_a edge distance of anchor channel, measured from edge of concrete member to axis of the nearest anchor as shown in Figure 20, in. (mm)

c_{a1} edge distance of anchor channel in direction 1 as shown in Figure 20, in. (mm)

c'_{a1} net distance between edge of the concrete member and the anchor channel: $c'_{a1} = c_{a1} - b_{ch}/2$, in. (mm)

$c_{a1,red}$ reduced edge distance of the anchor channel, as referenced in Eq. (17.7.2.7.7)

c_{a2} edge distance of anchor channel in direction 2 as shown in Figure 20, in. (mm)

$c_{a,max}$ maximum edge distance of anchor channel, in. (mm)

$c_{a,min}$ minimum edge distance of anchor channel, in. (mm)

c_{ac} edge distance required to develop full concrete capacity in absence of reinforcement to control splitting, in. (mm)

c_{cr} edge distance required to develop full concrete capacity in absence of anchor reinforcement, in. (mm)

$c_{cr,N}$ critical edge distance for anchor channel for tension loading for concrete breakout, in. (mm)

$c_{cr,Nb}$ critical edge distance for anchor channel for tension loading, concrete blow out, in. (mm)

$c_{cr,V}$ critical edge distance for anchor channel for shear loading, concrete edge breakout, in. (mm)

c_{nom} nominal concrete cover according to code, in. (mm)

d_1 width of head of I-anchors or diameter of head of round anchor, as shown in Figure 20 of this annex, in. (mm)

d_2 shaft diameter of round anchor, as shown in Figure 21 of this annex, in. (mm)

d_a diameter of anchor reinforcement, in. (mm)

d_s diameter of channel bolt, in. (mm)

e_1 distance between shear load and concrete surface, in. (mm)

e_s distance between the axis of the shear load and the axis of the anchor reinforcement resisting the shear load,
in. (mm)

f distance between anchor head and surface of the concrete, in. (mm)

f'_c specified concrete compressive strength, psi (MPa)

f_{uta} specified ultimate tensile strength of anchor, psi (MPa)

f_{utc} specified ultimate tensile strength of channel, psi (MPa)

f_{utb} specified ultimate tensile strength of channel bolt, psi (MPa)

f_y specified yield tensile strength of steel, psi (MPa)

f_{ya} specified yield strength of anchor, psi (MPa)

f_{yc} specified yield strength of channel, psi (MPa)

f_{yb} specified yield strength of channel bolt, psi (MPa)

h thickness of concrete member, as shown in Figure 20, in. (mm)

h_{ch} height of channel, as shown in Figure 20, in. (mm)

$h_{cr,V}$ critical member thickness, in. (mm)

h_{ef} effective embedment depth, as shown in Figure 20, in. (mm)

k load distribution factor, as referenced in Eq. (17.2.1.2.1.a)

k_{cp} pryout factor

l lever arm of the shear force acting on the channel bolt, in. (mm)

l_{in} influence length of an external load N_{ua} along an anchor channel, in. (mm)

ℓ_{dh} development length in tension of deformed bar or deformed wire with a standard hook, measured from critical section to outside end of hook, in. (mm)

ℓ_R length of deformed bar, in. (mm)

n_{ch} number of adjacent anchor channels

n_1 number of anchor rows in direction 1 perpendicular to the edge

p web thickness of I-anchor, as shown in Figure 21, in. (mm)

s spacing of anchors in direction of longitudinal axis of channel, in. (mm)

s_{chb} center-to-center distance between channel bolts in direction of longitudinal axis of channel, in. (mm)

$s_{ch,x}$ center-to-center spacing of adjacent end anchors of anchor channels in linear configuration, in. (mm)

$s_{ch,y}$ axis-to-axis spacing of two anchor channels in parallel configuration, in. (mm)

s_{cr} anchor spacing required to develop full concrete capacity in absence of anchor reinforcement, in. (mm)

$s_{cr,N}$ critical anchor spacing for tension loading, concrete breakout, in. (mm)

s_{max} maximum spacing of anchors of anchor channel, in. (mm)

s_{min} minimum spacing of anchors of anchor channel, in. (mm)

$s_{cr,Nb}$ critical anchor spacing for tension loading, concrete blow-out, in. (mm)

$s_{cr,V}$ critical anchor spacing for shear loading, concrete edge breakout, in. (mm)

w_A width of I-shaped anchor, as shown in Figure 20, in. (mm)

x distance between end of channel and nearest anchor, in. (mm)

z internal lever arm of the concrete member, in. (mm)

A_{brg} bearing area of anchor head, in.^{2} (mm^2)

A_i ordinate at the position of the anchor i, as illustrated in Figure 2, in. (mm)

$A_{se,N}$ effective cross-sectional area of anchor or channel bolt in tension, in.^{2} (mm^2)

$A_{se,V}$ effective cross-sectional area of channel bolt in shear, in.^{2} (mm^2)

I_y moment of inertia of the channel about principal y-axis, in.^{4} (mm^4)

M_1 bending moment on fixture around axis in direction 1, lbf-in. (Nm)

M_2 bending moment on fixture around axis in direction 2, lbf-in. (Nm)

$M_{s,flex}$ nominal flexural strength of the anchor channel, lbf-in. (Nm)

$M_{s,flex,allowable,ASD}$ allowable bending moment due to tension loads for use in allowable stress design environments,

lbf-in. (Nm)

M_{ss} flexural strength of the channel bolt, lbf-in. (Nm)

M^0_{ss} nominal flexural strength of the channel bolt, lbf-in. (Nm)

$M_{u,flex}$ bending moment on the channel due to tension loads, lbf-in. (Nm)

N_b basic concrete breakout strength of a single anchor in tension, lbf (N)

N_{ca} nominal strength of anchor reinforcement to take up tension loads, lbf (N)

N_{cb} concrete breakout strength of a single anchor of anchor channel in tension, lbf (N)

N_n lowest nominal tension strength of an anchor from all appropriate failure modes under tension, lbf (N)

N_p pullout strength of a single anchor of an anchor channel in tension, lbf (N)

N_{pn} nominal pullout strength of a single anchor of an anchor channel in tension, lbf (N)

N_{nc} nominal tension strength of one anchor from all concrete failure modes (lowest value of N_{cb} (anchor channels without anchor reinforcement to take up tension loads) or N_{ca} (anchor channels with anchor reinforcement to take up tension loads), N_{pn}, and N_{sb}), lbf (N)

N_{ns} nominal steel strength of anchor channel loaded in tension (lowest value of N_{sa}, N_{sc} and N_{sl}), lbf (N)

$N_{ns,a}$ nominal tension strength for steel failure of anchor or connection between anchor and channel (lowest value of N_{sa} and N_{sc}), lbf (N)

N_{sa} nominal tensile steel strength of a single anchor, lbf (N)

N_{sb} nominal concrete side-face blowout strength, lbf (N)

N^0_{sb} basic nominal concrete side-face blowout strength, lbf (N)

N_{sc} nominal tensile steel strength of the connection between anchor and channel profile, lbf (N)

N_{sl} nominal tensile steel strength of the local bending of the channel lips, lbf (N)

N_{ss} nominal tensile strength of a channel bolt, lbf (N)

N_{ua} factored tension load on anchor channel, lbf (N)

N^a_{ua} factored tension load on a single anchor of the anchor channel, lbf (N)

$N^a_{ua,i}$ factored tension load on anchor i of the anchor channel, lbf (N)

N^b_{ua} factored tension load on a channel bolt, lbf (N)

$N_{ua,re}$ factored tension load acting on the anchor reinforcement, lbf (N)

$T_{allowable,ASD}$ allowable tension load for use in allowable stress design environments, lbf (N)

T_{inst} Installation torque moment given in the manufacturer´s installation instruction, lbf-ft. (Nm)

$V_{x,allowable,ASD}$ allowable shear load longitudinal with the channel axis for use in allowable stress design environments, lbf (N)

$V_{y,allowable,ASD}$ allowable shear load perpendicular to the channel axis for use in allowable stress design environments, lb (N)V_b basic concrete breakout strength in shear of a single anchor, lbf (N)

$V_{ca,x}$ nominal strength of the anchor reinforcement of one anchor to take up shear loads longitudinal with the channel axis, lb (N)

$V_{ca,y}$ nominal strength of the anchor reinforcement of one anchor to take up shear loads perpendicular to the channel axis, lbf (N)

$V_{ca,y,max}$ maximum value of $V_{ca,y}$ of one anchor to be used in design, lbf (N)

$V_{cb,x}$ nominal concrete breakout strength in shear longitudinal with the channel axis of an anchor channel, lb (N)

$V_{cb,y}$ nominal concrete breakout strength in shear perpendicular to the channel axis of an anchor channel, lbf (N)

V_{cp} nominal pryout strength of a single anchor, lbf (N)

$V_{cp,x}$ nominal pry-out strength longitudinal with the channel axis of a single anchor, lb (N)

$V_{cp,y}$ nominal pryout strength perpendicular to the channel axis of a single anchor, lbf (N)

$V_{n,x}$ lowest nominal steel strength from all appropriate failure modes under shear longitudinal with the channel axis, lb (N)

$V_{n,y}$ lowest nominal steel strength from all appropriate failure modes under shear perpendicular to the channel axis, lbf (N)

Vnc nominal shear strength of one anchor from all concrete failure modes (lowest value of *Vcb* (anchor channels with anchor reinforcement to take up shear loads) or *Vca* (anchor channels with anchor reinforcement to take up shear loads) and *Vcp*), lbf (N)

Vns nominal steel strength of anchor channel loaded in shear (lowest value of *Vsa*, *Vsc*, and *Vsl*), lbf (N)

Vns,a nominal shear strength for steel failure of anchor or connection between anchor and channel (lowest value of *Vsa* and *Vsc*), lbf (N)

$V_{sa,x}$ nominal shear steel strength longitudinal with the channel axis of a single anchor, lb (N)

Vsa,y nominal shear steel strength perpendicular to the channel axis of a single anchor, lbf (N)

$V_{sa,x,seis}$ nominal seismic shear steel strength longitudinal with the channel axis of a single anchor, lb (N)

$V_{sa,y,seis}$ nominal seismic shear steel strength perpendicular to the channel axis of a single anchor, lb (N)

$V_{sc,x}$ nominal shear strength longitudinal with the channel axis of connection between one anchor and the anchor channel, lb (N)

Vsc,y nominal shear strength of connection between one anchor bolt and the anchor channel, lbf (N)

$V_{sc,x,seis}$ nominal seismic shear strength longitudinal with the channel axis of connection between one anchor bolt and the anchor channel, lb (N)

$V_{sc,y,seis}$ nominal seismic shear strength perpendicular to the channel axis of connection between one anchor bolt and the anchor channel, lb (N)

$V_{sl,x}$ nominal shear steel strength longitudinal with the channel axis of the local bending of the channel lips, lb (N)

$V_{sl,y}$ nominal shear steel strength perpendicular to the channel axis of the local bending of the channel lips, lbf (N)

$V_{sl,x,seis}$ nominal seismic shear steel strength longitudinal with the channel axis of the local bending of the channel lips, lb (N)

$V_{sl,y,seis}$ nominal seismic shear steel strength perpendicular to the channel axis of the local bending of the channel lips, lb (N)

V_{ss} nominal strength of channel bolt in shear, lbf (N)

$V_{ss,M}$ nominal strength of channel bolt in case of shear with lever arm, lbf (N)

V_{ua} factored shear load on anchor channel, lbf (N)

$V_{ua,x}$ factored shear load on anchor channel longitudinal with the channel axis, lb (N)

$V_{ua,y}$ factored shear load on anchor channel perpendicular to the channel axis, lbf (N)

V^a_{ua} factored shear load on a single anchor of the anchor channel, lbf (N)
$V^a_{ua,x}$ factored shear load on a single anchor of the anchor channel longitudinal with the channel axis, lb (N
$V^a_{ua,y}$ factored shear load on a single anchor of the anchor channel perpendicular to the channel axis, lbf (N)

$V^a_{ua,i}$ factored shear load on anchor i of the anchor channel, lbf (N)

$V^a_{ua,x,i}$ factored shear load on anchor i of the anchor channel in longitudinal channel axis, lb (N)

$V^a_{ua,y,i}$ factored shear load on anchor i of the anchor channel perpendicular to the channel axis, lbf (N)

V^b_{ua} factored shear load on a channel bolt, lbf (N)

$V^b_{ua,x}$ factored shear load on a channel bolt in longitudinal channel axis, lb (N)

$V^b_{ua,y}$ factored shear load on a channel bolt perpendicular to the channel axis, lbf (N)

α exponent of interaction equation [-]

α_{ASD} conversion factor for allowable stress design [-]

$\alpha_{ch,N}$ factor to account for the influence of channel size on concrete breakout strength in tension [-]

α_M factor to account for the influence of restraint of fixture on the flexural strength of the channel bolt [-]

$\alpha_{ch,V}$ factor to account for the influence of channel size and anchor diameter on concrete edge breakout strength in shear, ($lbf^{1/2}/in.^{1/3}$) ($N^{1/2}/mm^{1/3}$)

$\psi_{c,N}$ modification factor to account for influence of cracked or uncracked concrete on concrete breakout strength [-]

$\psi_{c,Nb}$ modification factor to account for influence of cracked or uncracked concrete on concrete blowout strength [-]

$\psi_{c,V}$ modification factor to account for influence of cracked or uncracked concrete for concrete edge breakout strength [-]

$\psi_{co,N}$ modification factor for corner effects on concrete breakout strength for anchors loaded in tension [-]

$\psi_{co,Nb}$ modification factor for corner effects on concrete blowout strength for anchors loaded in tension [-]

$\psi_{co,V}$ modification factor for corner effects on concrete edge breakout strength for anchor channels loaded in shear [-]

$\psi_{cp,N}$ modification factor for anchor channels to control splitting [-]

$\psi_{ed,N}$ modification factor for edge effect on concrete breakout strength for anchors loaded in tension [-]

$\psi_{g,Nb}$ modification factor to account for influence of bearing area of neighboring anchors on concrete blowout strength for anchors loaded in tension [-]

$\psi_{h,Nb}$ modification factor to account for influence of member thickness on concrete blowout strength for anchors loaded in tension [-]

$\psi_{h,V}$ modification factor to account for influence of member thickness on concrete edge breakout strength for anchors channels loaded in shear [-]

$\psi_{s,N}$ modification factor to account for influence of location and loading of neighboring anchors on concrete breakout strength for anchor channels loaded in tension [-]

$\psi_{s,V}$ modification factor to account for influence of location and loading of neighboring anchors on concrete edge breakout strength for anchor channels loaded in shear [-]

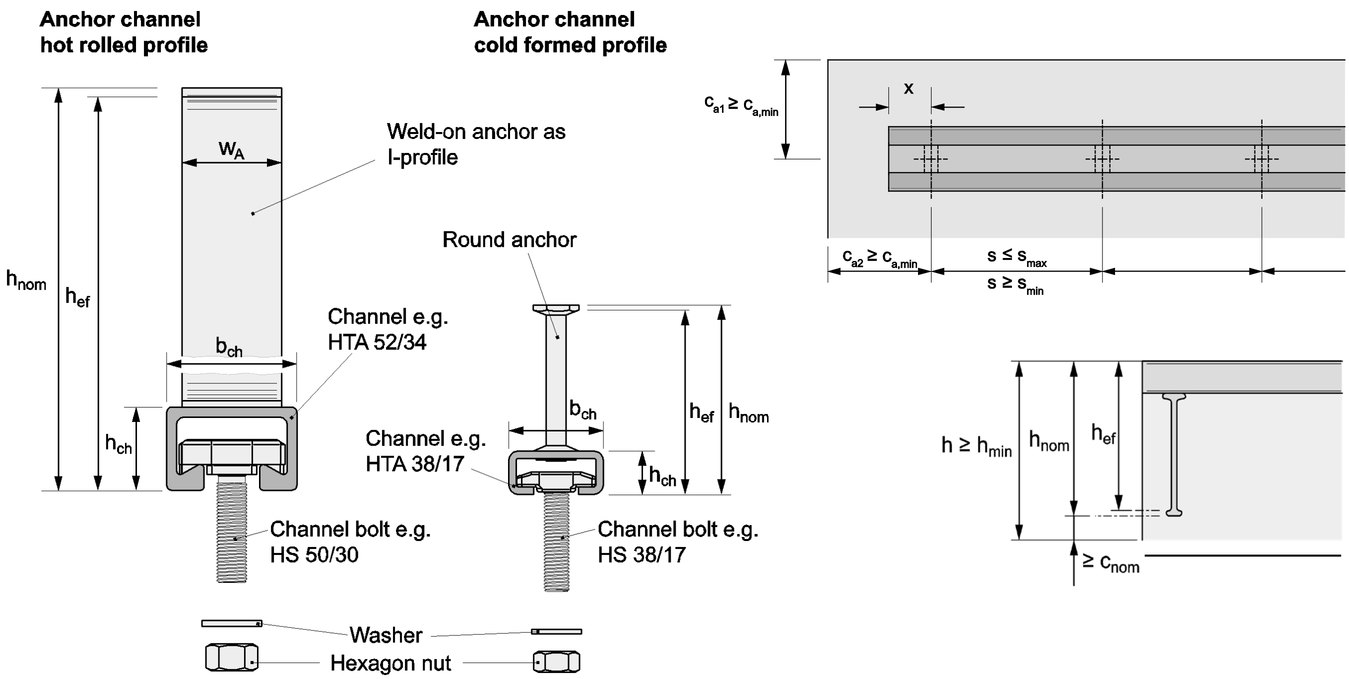


FIGURE 20—INSTALLATION PARAMETERS FOR ANCHOR CHANNELS

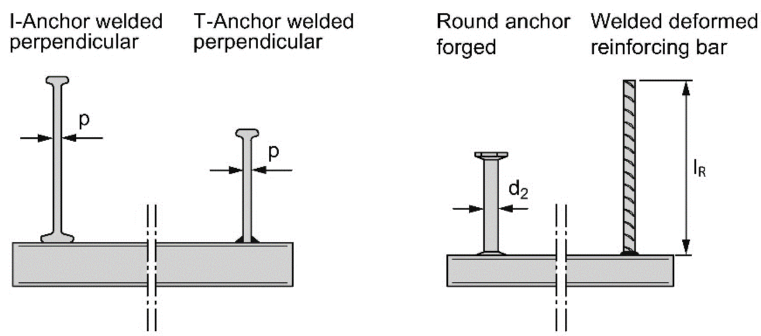


FIGURE 21—TYPES OF ANCHORS

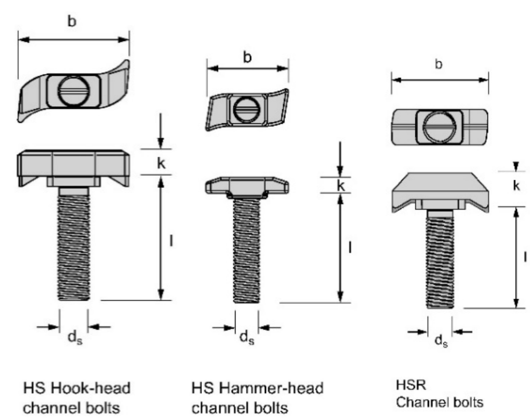


FIGURE 22—CHANNEL BOLTS

TABLE 1—INSTALLATION PARAMETERS FOR HALFEN HTA ANCHOR CHANNELS

CRITERIA	SYMBOL	UNITS	ANCHOR CHANNEL SIZES							
			28/15 [1)]	38/17 [1)]	41/22 [1)]	40/22	50/30	52/34	55/42	72/48
Channel height	h_{ch}	in.	0.60	0.69	0.81	0.91	1.18	1.32	1.65	1.91
		(mm)	(15.25)	(17.5)	(20.7)	(23.0)	(30.0)	(33.5)	(42.0)	(48.5)
Channel width	b_{ch}	in.	1.10	1.50	1.63	1.56	1.93	2.07	2.15	2.83
		(mm)	(28.0)	(38.0)	(41.3)	(39.5)	(49.0)	(52.5)	(54.5)	(72.0)
Moment of inertia, carbon and stainless steel	I_y	$in.^4$	0.0098	0.0205	0.031	0.0481	0.1271	0.2241	0.4504	0.8402
		(mm^4)	(4,060)	(8,547)	(12,910)	(20,029)	(52,896)	(93,262)	(187,464)	(349,721)
Minimum anchor spacing	s_{min}	in.	1.97	1.97	1.97	1.97	1.97	3.15	3.15	3.15
		(mm)	(50)	(50)	(50)	(50)	(50)	(80)	(80)	(80)
Minimum anchor spacing, welded reinforcing bars	s_{min}	in.	-	-	-	4.00	4.00	4.00	4.00	-
		(mm)	-	-	-	(100)	(100)	(100)	(100)	-
Maximum anchor spacing	s_{max}	in.	7.87	7.87	9.84	9.84	9.84	9.84	11.81	15.75
		(mm)	(200)	(200)	(250)	(250)	(250)	(250)	(300)	(400)
Installation height, round anchor	h_{nom}	in.	1.86	3.07	3.31	3.67	4.28	-	-	-
		(mm)	(47.25)	(77.9)	(84)	(93.2)	(108.7)	-	-	-
Installation height, welded I-shaped anchors	h_{nom}	in.	3.04	3.13	-	3.62	3.90	6.36	7.17	7.42
		(mm)	(77.25)	(79.5)	-	(92.0)	(99.0)	(161.5)	(182.0)	(188.5)
Installation height, welded T-shaped anchors	h_{nom}	in.	-	-	-	-	3.25	3.74	4.72	-
		(mm)	-	-	-	-	(82.5)	(95.0)	(120.0)	-
Reinforcing bar size	d_b	-	-	-	-	#4	#4	#5	#5	-
Length of deformed reinforcing bar	ℓ_R	in.	-	-	-	acc. ACI 318-11 Sec. 12.14 or ACI 318-14 Sec. 25.5				-
		(mm)	-	-	-					-
Minimum edge distance	$c_{a,min}$	in.	1.57	1.97	1.97	1.97	2.95	2.95	3.94	5.91
		(mm)	(40)	(50)	(50)	(50)	(75)	(75)	(100)	(150)
End spacing	x	in.	0.98	0.98	0.98	0.98	0.98	1.38	1.38	1.38
		(mm)	(25)	(25)	(25)	(25)	(25)	(35)	(35)	(35)
Minimum shaft diameter	d_2	in.	0.24	0.31	0.31	0.39	0.47	-	-	-
		(mm)	(6.0)	(8.0)	(8.0)	(10.0)	(12.0)	-	-	-
Minimum web thickness	p	in.	0.20	0.20	-	0.20	0.20	0.24	0.28	0.28
		(mm)	(5.0)	(5.0)	-	(5.0)	(5.0)	(6.0)	(7.1)	(7.1)
Minimum width of I- or T-shaped anchors	w_A	in.	0.59	0.59	-	1.00	1.18	1.54	1.57	1.97
		(mm)	(15.0)	(15.0)	-	(25.4)	(30.0)	(39.0)	(40.0)	(50.0)
Min member thickness, round anchors	h_{min}	in.	2.49	3.70	4.92	4.30	4.91	-	-	-
		(mm)	(63.2)	(93.9)	(125)	(109.2)	(124.7)	-	-	-
Min member thickness, welded I-shaped anchors	h_{min}	in.	3.67	3.76	-	4.25	4.53	6.99	7.80	8.05
		(mm)	(93.2)	(95.5)	-	(108.0)	(115.0)	(177.5)	(198.0)	(204.5)
Min member thickness, welded T-shaped anchors	h_{min}	in.	-	-	-	-	4.00	4.375	5.19	-
		(mm)	-	-	-	-	(101.6)	(111.1)	(131.8)	-

For **SI:** 1 in. = 25.4 mm
For inch-pound units: 1 mm = 0.03937 in.
[1)] Carbon and stainless steel

TABLE 2—COMBINATION ANCHOR CHANNEL – CHANNEL BOLTS

CRITERIA	SYMBOL	UNITS	ANCHOR CHANNEL SIZES								
			28/15	38/17	41/22	40/22		50/30	52/34	55/42	72/48
Bolt type			HS 28/15 [1]	HS 38/17 [1]	HZS 41/22 [1]	HS 40/22 [2]	HSR 40/22 [3]	HS 50/30 [2]	HS 50/30 [2]	HS 50/30 [2]	HS 72/48 [2]
Diameter	d_s	(mm)	(8)	-	-	-	-	-	-	-	-
		(mm)	(10)	(10)	-	(10)	-	(10)	(10)	(10)	-
		(mm)	(12)	(12)	(12)	(12)	-	(12)	(12)	(12)	-
		(mm)	-	(16)	(16)	(16)	(16)	(16)	(16)	(16)	-
		(mm)	-	-	-	-	-	(20)	(20)	(20)	(20)
		(mm)	-	-	-	-	-	-	-	(24)	(24)
		(mm)	-	-	-	-	-	-	-	-	(27)
		(mm)	-	-	-	-	-	-	-	-	(30)

For **SI:** 1 in. = 25.4 mm

Notes for Table 2:

For inch-pound units: 1 mm = 0.03937 in.
[1] Hammer-head channel bolts
[2] Hook-head channel bolts
[3] Locking channel bolts

TABLE 3—HTA ANCHOR CHANNELS: STEEL STRENGTH IN TENSION

CRITERIA	SYMBOL	UNITS	ANCHOR CHANNEL SIZES							
			28/15	38/17	41/22	40/22	50/30	52/34	55/42	72/48
Nominal strength for local bending of channel lips, tension	N_{sl}	lbf	2,025	4,045	4,069	6,745	8,770	14,615	21,355	26,975
		(kN)	(9.0)	(18.0)	(18.1)	(30.0)	(39.0)	(65.0)	(95.0)	(120.0)
Nominal strength for local bending of channel lips in tension for seismic design	$N_{sl,.seis}$	lbf	-	-	-	6,745	-	-	-	-
		(kN)	-	-	-	(30.0)	-	-	-	-
Nominal steel strength of a single anchor in tension, round anchors	N_{sa}	lbf	2,045	4,520	4,520	7,060	12,700	-	-	-
		(kN)	(9.1)	(20.1)	(20.1)	(31.4)	(56.5)	-	-	-
Nominal steel strength of a single anchor in tension,	$N_{sa,seis}$	lbf	-	-	-	7,060	-	-	-	-
		(kN)	-	-	-	(31.4)	-	-	-	-
Nominal steel strength of a single anchor in tension, welded I- or T-shaped anchors	N_{sa}	lbf	6,070	6,070	-	10,275	12,140	18,930	22,975	28,730
		(kN)	(27.0)	(27.0)	-	(45.7)	(54.0)	(84.2)	(102.2)	(127.8)
Nominal steel strength of a single anchor in tension, welded I- or T-shaped anchors for seismic design	$N_{sa,seis}$	lbf	-	-	-	10,275	-	-	-	-
		(kN)	-	-	-	(45.7)	-	-	-	-
Nominal steel strength of a single reinforcing bar in tension	N_{sa}	lbf	-	-	-	16,000	16,000	24,800	24,800	-
		(kN)	-	-	-	(71.2)	(71.2)	(110.3)	(110.3)	-
Nominal steel strength of a single reinforcing bar in tension for seismic design	$N_{sa,seis}$	lbf	-	-	-	16,000	-	-	-	-
		(kN)	-	-	-	(71.2)	-	-	-	-
Nominal tension strength connection channel / anchor	N_{sc}	lbf	2,025	4,045	4,069	6,520	8,770	14,615	17,985	24,280
		(kN)	(9.0)	(18.0)	(18.1)	(29.0)	(39.0)	(65.0)	(80.0)	(108.0)
Nominal tension strength connection channel / anchor for seismic design	$N_{sc,seis}$	lbf	-	-	-	6,520	-	-	-	-
		(kN)	-	-	-	(29.0)	-	-	-	-
Nominal tension strength connection channel / reinforcing bar	N_{sc}	lbf	-	-	-	6,670	8,145	17,095	19,695	-
		(kN)	-	-	-	(29.7)	(36.2)	(76.0)	(87.6)	-
Nominal tension strength connection channel / reinforcing bar for seismic design	$N_{sc,seismic}$	lbf	-	-	-	6,670	-	-	-	-
		(kN)	-	-	-	(29.7)	-	-	-	-
Strength reduction factor[1]	ϕ	-	0.75							
Nominal bending strength, carbon steel	$M_{s,flex}$	lbf-in.	2,745	5,135	6,488	11,825	19,835	32,550	54,260	77,725
		(Nm)	(310)	(580)	(733)	(1,336)	(2,241)	(3,678)	(6,131)	(8,782)
Nominal bending strength, carbon steel for seismic design	$M_{s,flex,seismic}$	lbf-in.	-	-	-	11,825	-	-	-	-
		(Nm)	-	-	-	(1,336)	-	-	-	-
Nominal bending strength, stainless steel	$M_{s,flex}$	lbf-in.	2,830	5,250	6,629	-	-	-	-	-
		(Nm)	(320)	(593)	(749)	-	-	-	-	-
Strength reduction factor[1]	ϕ	-	0.85							

For **SI:** 1 in. = 25.4 mm, 1 lbf = 4.448 N, 1 lbf-in. = 8.85 Nm
For inch-pound units: 1 mm = 0.03937 in., 1 N = 0.2248 lbf, 1 Nm = 0.113 lbf-in.

[1] The tabulated value of ϕ applies when the load combinations of Section 1605.1 of the 2024 IBC or ACI 318-19 Section 5.3 are used.

TABLE 4—HTA ANCHOR CHANNELS: CONCRETE STRENGTH IN TENSION

CRITERIA	SYMBOL	UNITS	ANCHOR CHANNEL SIZES							
			28/15	38/17	41/22	40/22	50/30	52/34	55/42	72/48
Effective embedment depth for round and/or I-shaped anchors[2]	h_{ef}	in.	1.77	2.99	3.23	3.50	3.78	6.10	6.89	7.05
		(mm)	(45)	(76)	(82.1)	(89)	(96)	(155)	(175)	(179)
Effective embedment depth for welded T-shaped anchors	h_{ef}	in.	-	-	-	-	3.11	3.54	4.49	-
		(mm)	-	-	-	-	(79)	(90)	(114)	-
Area of anchor head, round anchor	A_{brg}	$in.^2$	0.13	0.23	0.23	0.37	0.59	-	-	-
		(mm^2)	(84.8)	(150.8)	(150.8)	(235.6)	(377.8)	-	-	-
Area of anchor head, welded I- or T-shaped anchors	A_{brg}	$in.^2$	0.30	0.30	-	0.51	0.60	0.66	0.80	1.00
		(mm^2)	(195.0)	(195.0)	-	(330.2)	(390.0)	(429.0)	(516.0)	(645.0)
Critical edge distance	c_{ac}	in. (mm)	$c_{ac} = 3 \cdot h_{ef}$							
Strength reduction factor[1]	ϕ	-	0.70							

For **SI:** 1 in. = 25.4 mm, 1 lbf = 4.448 N

For inch-pound units: 1 mm = 0.03937 in., 1 N = 0.2248 lbf

[1] The tabulated value of ϕ applies when both the load combinations of Section 1605.1 of the 2024 IBC or ACI 318-19 Section 5.3 are used and the requirements of ACI 318-19 Table 17.5.3(b) for supplementary reinforcement not present are met. For installations where complying supplementary reinforcement can be verified, the ϕ factors described in ACI 318-19 Table 17.5.3(b) are allowed.

[2] Embedment depth value is for design calculation used for round anchor and welded anchor, refer to table 1 for required, h_{nom}, installation embedment.

TABLE 5—HTA ANCHOR CHANNELS: STEEL STRENGTH IN SHEAR

CRITERIA	SYMBOL	UNITS	ANCHOR CHANNEL SIZES							
			28/15	38/17	41/22	40/22	50/30	52/34	55/42	72/48
Nominal strength for local bending of channel lips, perpendicular shear	$V_{sl,y}$	lbf	2,025	4,045	4,069	6,745	10,115	15,735	22,480	26,975
		(kN)	(9.0)	(18.0)	(18.1)	(30.0)	(45.0)	(70.0)	(100.0)	(120.0)
Nominal strength for local bending of channel lips, perpendicular shear for seismic design	$V_{sl,y,seis}$	lbf	-	-	-	6,745	-	-	-	-
		(kN)	-	-	-	(30.0)	-	-	-	-
Strength reduction factor[1]	ϕ	-	0.75							
Nominal strength for local bending of channel lips, longitudinal shear	$V_{sl,x}$	lbf	-	-	-	3,025	-	-	-	-
		(kN)	-	-	-	(13.5)	-	-	-	-
Nominal strength for local bending of channel lips, longitudinal shear for seismic design	$V_{sl,x,seis}$	lbf	-	-	-	3,025	-	-	-	-
		(kN)	-	-	-	(13.5)	-	-	-	-
Strength reduction factor[1] (periodic inspection)	ϕ	-	0.65							
Strength reduction factor[1] (continuous inspection)	ϕ	-	0.75							
Nominal steel strength of a single anchor in shear, round anchors	$V_{sa,y}$	lbf	2,025	4,045	4,069	6,745	10,115	-	-	-
		(kN)	(9.0)	(18.0)	(18.1)	(30.0)	(45.0)	-	-	-
Nominal steel strength of a single anchor in shear, round anchors for seismic design	$V_{sa,y,seis}$	lbf	-	-	-	6,745	-	-	-	-
		(kN)	-	-	-	(30.0)	-	-	-	-
Nominal steel strength of a single anchor in shear, welded I- or T-Shaped anchors	$V_{sa,y}$	lbf	2,025	4,045	-	6,745	10,115	15,735	22,480	26,975
		(kN)	(9.0)	(18.0)	-	(30.0)	(45.0)	(70.0)	(100.0)	(120.0)
Nominal steel strength of a single anchor in shear, welded I- or T-Shaped anchors for seismic design	$V_{sa,y,seis}$	lbf	-	-	-	6,745	-	-	-	-
		(kN)	-	-	-	(30.0)	-	-	-	-
Nominal steel strength of a single welded reinforcing bar	$V_{sa,y}$	lbf	-	-	--	6,670	8,145	15,735	19,695	-
		(kN)	-	-	-	(29.7)	(36.2)	(70.0)	(87.6)	-
Nominal steel strength of a single welded reinforcing bar for seismic design	$V_{sa,y,seis}$	lbf	-	-	-	6,670	-	-	-	-
		(kN)	-	-	-	(29.7)	-	-	-	-
Nominal steel strength of a single anchor in shear, round anchors	$V_{sa,x}$	lbf	-	-	-	4,235	-	-	-	-
		(kN)	-	-	-	(18.8)	-	-	-	-
Nominal steel strength of a single anchor in shear, round anchors for seismic design	$V_{sa,x,seis}$	lbf	-	-	-	4,235	-	-	-	-
		(kN)	-	-	-	(18.8)	-	-	-	-
Nominal steel strength of a single anchor in shear, welded I- or T-Shaped anchors	$V_{sa,x}$	lbf	-	-	-	6,165	-	-	-	-
		(kN)	-	-	-	(27.4)	-	-	-	-
Nominal steel strength of a single anchor in shear, welded I- or T-Shaped anchors for seismic design	$V_{sa,x,seis}$	lbf	-	-	-	6,165	-	-	-	-
		(kN)	-	-	-	(27.4)	-	-	-	-
Nominal steel strength of a single welded reinforcing bar	$V_{sa,x}$	lbf	-	-	-	9,605	-	-	-	-
		(kN)	-	-	-	(42.7)	-	-	-	-
Nominal steel strength of a single welded reinforcing bar for seismic design	$V_{sa,x,seis}$	lbf	-	-	-	9,605	-	-	-	-
		(kN)	-	-	-	(42.7)	-	-	-	-
Nominal shear strength for connection channel / anchor	$V_{sc,y}$	lbf	2,025	4,045	4,069	6,745	10,115	15,735	22,480	26,975
		(kN)	(9.0)	(18.0)	(18.1)	(30.0)	(45.0)	(70.0)	(100.0)	(120.0)
Nominal shear strength for connection channel / anchor for seismic design	$V_{sc,y,seis}$	lbf	-	-	-	6,745	-	-	-	-
		(kN)	-	-	-	(30.0)	-	-	-	-
Nominal shear strength for connection channel / reinforcing bar	$V_{sc,y}$	lbf	-	-	-	6,670	8,145	15,735	19,695	-
		(kN)	-	-	-	(29.7)	(36.2)	(70.0)	(87.6)	-
Nominal shear strength for connection channel / reinforcing bar for seismic design	$V_{sc,y,seis}$	lbf	-	-	-	6,670	-	-	-	-
		(kN)	-	-	-	(29.7)	-	-	-	-
Nominal shear strength for connection channel / anchor	$V_{sc,x}$	lbf	-	-	-	3,910	-	-	-	-
		(kN)	-	-	-	(17.4)	-	-	-	-
Nominal shear strength for connection channel / anchor for seismic design	$V_{sc,x,seis}$	lbf	-	-	-	3,910	-	-	-	-
		(kN)	-	-	-	(17.4)	-	-	-	-
Nominal shear strength for connection channel / reinforcing bar	$V_{sc,x}$	lbf	-	-	-	4,005	-	-	-	-
		(kN)	-	-	-	(17.8)	-	-	-	-
Nominal shear strength for connection channel / reinforcing bar for seismic design	$V_{sc,x,seis}$	lbf	-	-	-	4,005	-	-	-	-
		(kN)	-	-	-	(17.8)	-	-	-	-
Strength reduction factor[1]	ϕ	-	0.75							

For **SI:** 1 lbf = 4.448 N
For inch-pound units: 1 N = 0.2248 lbf

[1] The tabulated value of ϕ applies when the load combinations of Section 1605.1 of the 2024 IBC or ACI 318-19 Section 5.3 are used.

TABLE 6—HTA ANCHOR CHANNELS: STATIC CONCRETE STRENGTH IN SHEAR

CRITERIA	SYMBOL	UNITS	ANCHOR CHANNEL SIZES							
			28/15	38/17	41/22	40/22	50/30	52/34	55/42	72/48
Cracked concrete without reinforcement	$\alpha_{ch,V}$	$lbf^{1/2}/in.^{1/3}$	5.6	10.5	9.1	10.5	10.5	10.5	10.5	10.5
		$(N^{1/2}/mm^{1/3})$	(4.0)	(7.5)	(6.5)	(7.5)	(7.5)	(7.5)	(7.5)	(7.5)
Pryout failure, factor	k_{cp}	-	1.0	2.0						
Strength reduction factor[1]	ϕ	-	0.70							

[1] The tabulated value of ϕ applies when both the load combinations of Section 1605.1 of the 2024 IBC or ACI 318-19 Section 5.3 are used and the requirements of ACI 318-19 Table 17.5.3(b) for supplementary reinforcement not present are met.. For installations where complying supplementary reinforcement can be verified, the ϕ factors described in ACI 318-19 Table 17.5.3(b) are allowed.

TABLE 7—HS / HZS / HSR CHANNEL BOLTS: STATIC STEEL STRENGTH IN TENSION

CRITERIA	SYMBOL	UNITS	GRADE / MATERIAL	BOLT	CHANNEL BOLT SIZES							
					M8	M10	M12	M16	M20	M24	M27	M30
Nominal tensile strength	N_{ss}	lbf (Kn)	4.6	28/15	2,967	4,181	4,946		-	-	-	-
					(13.2)	(18.6)	(22.0)		-	-	-	-
				38/17	-	5,216	6,160	12,342	-	-	-	-
					-	(23.2)	(27.4)	(54.9)	-	-	-	-
				40/22	-	5,216	7,576	13,466	-	-	-	-
					-	(23.2)	(33.7)	(59.9)	-	-	-	-
				50/30	-	5,216	7,576	14,118	22,031	31,743	-	-
					-	(23.2)	(33.7)	(62.8)	(98.0)	(141.2)	-	-
				72/48	-	-	-	-	22,031	31,743	41,275	50,447
					-	-	-	-	(98.0)	(141.2)	(183.6)	(224.4)
			8.8	28/15	-	-	-	-	-	-	-	-
				38/17	-	-	13,129	20,817	-	-	-	-
					-	-	(58.4)	(92.6)	-	-	-	-
				41/22	-	-	10,903	21,649	-	-	-	-
					-	-	(48.5)	(96.3)	-	-	-	-
				40/22[2]	-	10,431	15,152	28,236	-	-	-	-
					-	(46.4)	(67.4)	(125.6)	-	-	-	-
				50/30	-	10,431	15,152	28,236	44,063	63,486	-	-
					-	(46.4)	(67.4)	(125.6)	(196.0)	(282.4)	-	-
				72/48	-	-	-	-	44,063	63,486	82,550	-
					-	-	-	-	(196.0)	(282.4)	(367.2)	-
			Stainless steel grade 50	28/15	-	-	-	-	-	-	-	-
				38/17	-	-	-	11,196	-	-	-	-
					-	-	-	(49.8)	-	-	-	-
				41/22	-	-	9,060	14,388	-	-	-	-
					-	-	(40.3)	(64)	-	-	-	-
			Stainless steel grade 70	28/15	5,755	9,127	-	-	-	-	-	-
					(25.6)	(40.6)	-	-	-	-	-	-
				38/17	-	9,127	9,914	-	-	-	-	-
					-	(40.6)	(44.1)	-	-	-	-	-
Nominal tensile strength for seismic design	$N_{ss,seis}$	lbf (Kn)	8.8	40/22[2]	-	-	-	28,236	-	-	-	-
					-	-	-	(125.6)	-	-	-	-
Strength reduction factor[1]	ϕ	-	4.6	-	0.75							
			8.8[2]		0.65							
			Grade 50		0.75							
			Grade 70		0.65							

[1] The tabulated value of ϕ applies when the load combinations of Section 1605.1 of the IBC or ACI 318-19 Section 5.3 are used.
[2] The tabulated value is also valid for HSR bolts.

TABLE 8—HS / HZS / HSR CHANNEL BOLTS: STATIC STEEL STRENGTH IN SHEAR

CRITERIA	SYMBOL	UNITS	GRADE / MATERIAL	CHANNEL BOLT SIZES							
				M8	M10	M12	M16[2]	M20	M24	M27	M30
Nominal shear strength	V_{ss}	lbf (kN)	4.6	1,980	3,125	4,540	8,475	13,220	19,040	24,775	30,260
				(8.8)	(13.9)	(20.2)	(37.7)	(58.8)	(84.7)	(110.2)	(134.6)
			8.8[2]	-	6,250	9,105	16,950	26,440	38,085	49,525	-
				-	(27.8)	(40.5)	(75.4)	(117.6)	(169.4)	(220.3)	-
			Stainless steel grade 50	-	-	4,535	8,455	-	-	-	-
				-	-	(20.2)	(37.6)	-	-	-	-
			Stainless steel grade 70	3,460	5,485	7,960	-	-	-	-	-
				(15.4)	(24.4)	(35.4)	-	-	-	-	-
Nominal shear strength for seismic design	$V_{ss,seis}$	lbf (kN)	8.8[2]	-	-	-	16,950	-	-	-	-
				-	-	-	(75.4)	-	-	-	-
Strength reduction factor for steel failure under shear[1]	ϕ	-	4.6	0.65							
			8.8	0.60							
			Grade 50	0.65							
			Grade 70	0.60							
Nominal bending strength	M^0_{ss}	lbf-in. (Nm)	4.6	133.0	265.0	464.0	1,180	2,300	3,975	5,890	7,960
				(15.0)	(29.9)	(52.4)	(133.2)	(259.6)	(449.0)	(665.8)	(899.6)
			8.8	-	529.0	927.0	2,360[3]	4,600	7,945	11,785	-
				-	(59.8)	(104.8)	(266.4)[3]	(519.3)	(898.0)	(1,331.5)	-
			Stainless steel grade 50	-	-	463.0	1,175	-	-	-	-
				-	-	(52.3)	(132.9)	-	-	-	-
			Stainless steel grade 70	232.0	463.0	812.0	-	-	-	-	-
				(26.2)	(52.3)	(91.7)	-	-	-	-	-
Nominal bending strength for seismic design	$M^0_{ss,seis}$	lbf-in. (Nm)	8.8[2]	-	-	-	2,360	-	-	-	-
				-	-	-	(266.4)	-	-	-	-
Strength reduction factor for bending failure[1]	ϕ	-	4.6	For bolts in shear where bending of the bolt occurs the strength reduction factor for bolts in tension in accordnace with Table 7 shall be used.							
			8.8								
			Grade 50								
			Grade 70								

For **SI:** 1 in. = 25.4 mm, 1 lbf = 4.448 N, 1 lbf-in. = 8.85 Nm
For inch-pound units: 1 mm = 0.03937 in., 1 N = 0.2248 lbf, 1 Nm = 0.113 lbf-in.

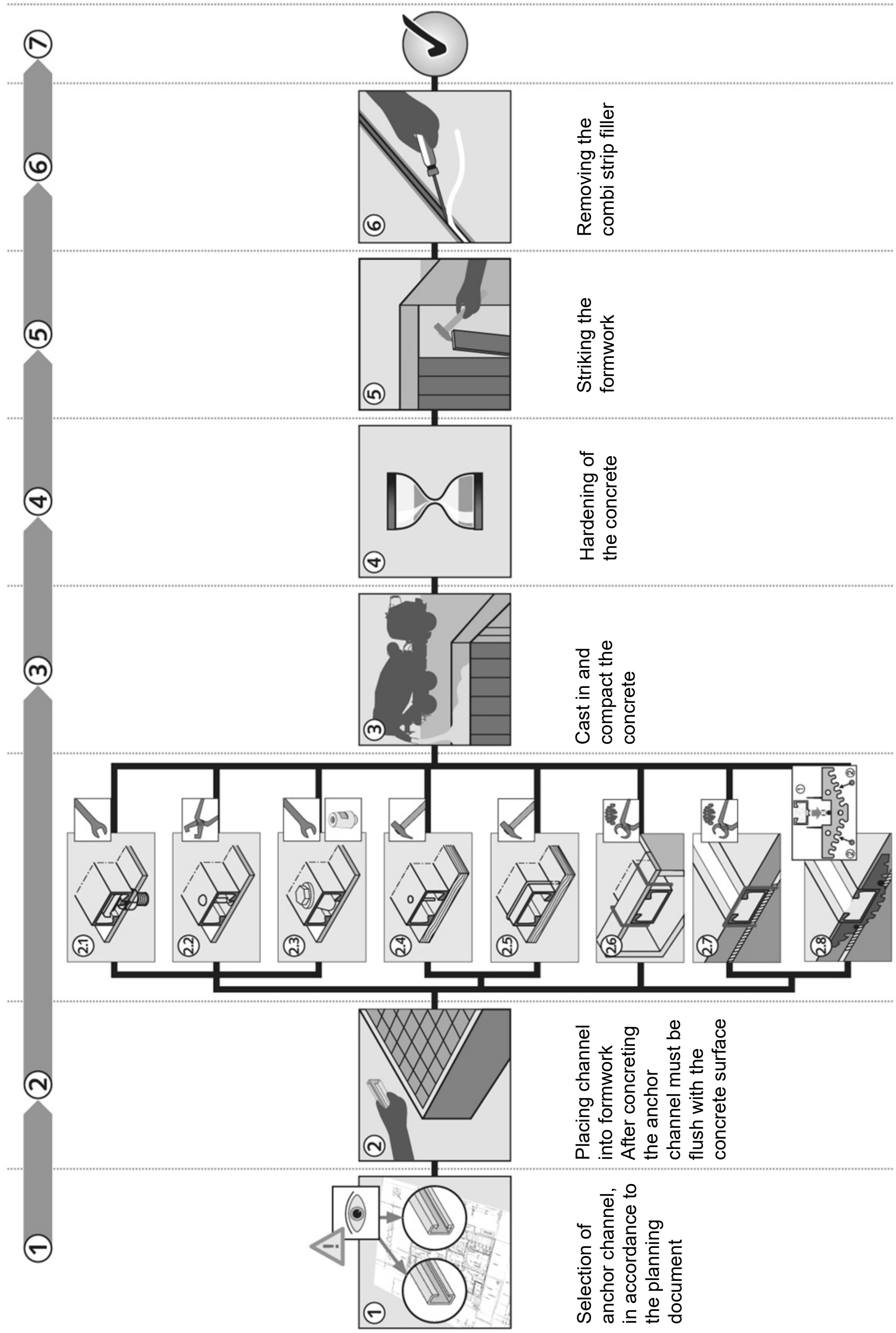
[1] The tabulated value of ϕ applies when the load combinations of Section 1605.1 of the IBC or ACI 318-19 Section 5.3 are used.
[2] The tabulated value is also valid for HSR bolts.
[3] For HZS 41/22 M16 8.8, M^0_{ss} is limited to 2,310 lbf-in (261 Nm).

TABLE 9—HTA ANCHOR CHANNELS AND HS CHANNEL BOLTS: MATERIAL SPECIFICATION AND PROPERTIES

COMPONENT	CARBON STEEL		STAINLESS STEEL
	Material / Strength class	Coating	Material / Strength class
Channel profile	Carbon steel	Hot-dip galvanized ≥ 55 μm	Stainless steel A4
Anchor	Carbon steel	Hot-dip galvanized ≥ 55 μm	Stainless steel A4
Deformed reinforcing bar	Low-alloy steel according to ASTM A705 or carbon steel according to DIN 488-BSt 500	-	-
Channel bolts	Carbon steel grade 4.6 and 8.8 according to EN ISO 898-1	Hot-dip galvanized ≥ 50 μm or electroplated ≥ 12 μm	Stainless steel grade 50 and 70 according to EN ISO 3506-1
Plain washer [1] ISO 7089 and ISO 7093-1	Production class A, 200 HV	Hot-dip galvanized or electroplated	Production class A, 200 HV according to EN ISO 3506-1
Hexagonal nuts ISO 4032	Property class 5 and 8 according to EN ISO 898-2	Hot-dip galvanized ≥ 50 μm or electroplated ≥ 12 μm	Stainless steel grade 70 and 80 according to EN ISO 3506-2

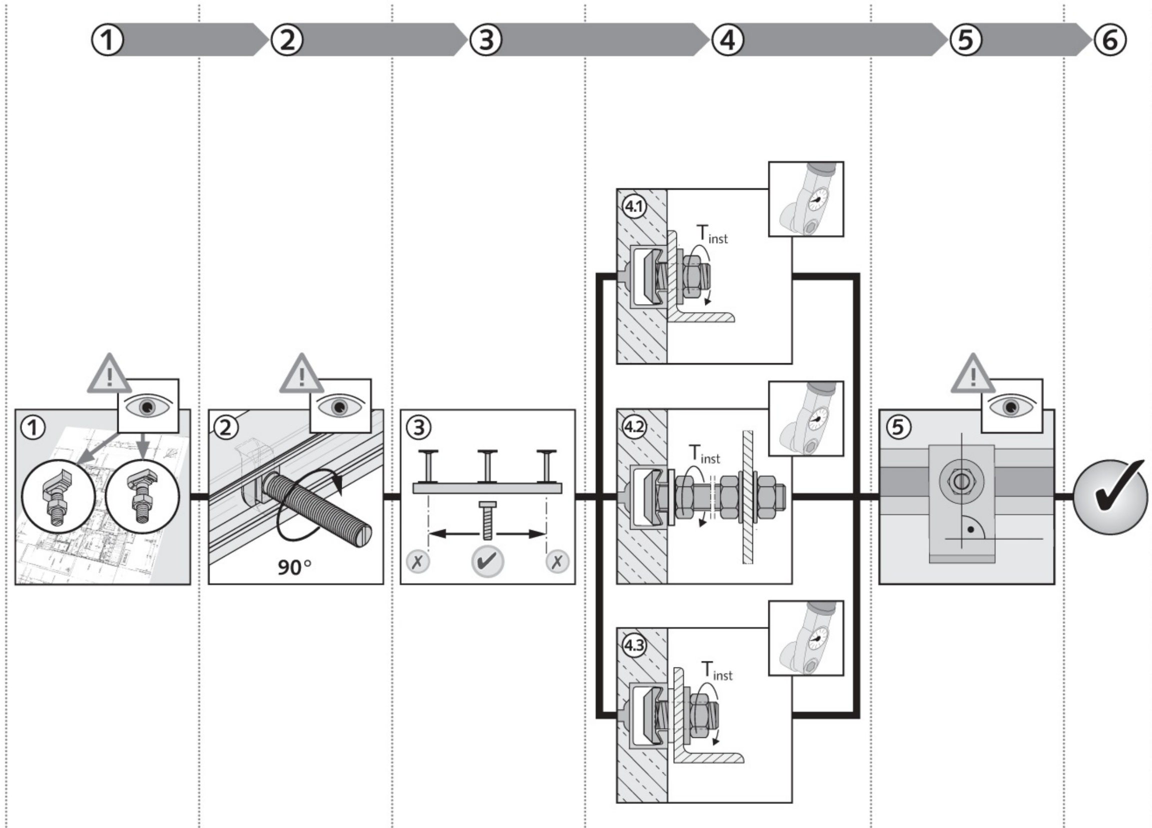
[1] Not in scope of delivery

TABLE 10—HTA ANCHOR CHANNELS : INSTALLATION INSTRUCTION



2.1 Steel formwork: Fixing with HALFEN channel bolts through formwork penetration
2.2 Steel formwork: Fixing with rivets
2.3 Steel formwork: Fixing with HALFEN fixing cone
2.4 Timber formwork: Fixing with nails
2.5 Timber formwork: Fixing with staples
2.6 Fixing in the top surface of concrete: Fixing by using auxiliary construction
2.7 Fixing in the top surface of concrete: Fixing from above directly to the reinforcement
2.8 Fixing in the top surface of concrete: Fixing to the reinforcement, using the HALFEN ChanClip

TABLE 11—HS / HSR CHANNEL BOLTS : INSTALLATION INSTRUCTION



①	②	③	④	⑤	⑥
Selection of the HALFEN channel bolts in accordance with the construction document.	Insert the channel bolt into the channel. After a 90° turn clockwise, the channel bolt locks into the channel. (Check of the position of the bolt by notch).	Positioning of the channel bolt: At the channel ends a minimum clearance must be maintained, which corresponds with the overhang beyond the last anchor.	Tighten the hexagonal nut to the setting torque (T_{inst}) acc. table stated below. T_{inst} must not be exceeded. 4.1: general 4.2 and 4.3: steel to steel contact.	After fixing the nuts, check the correct position of the bolt: If the notch is not perpendicular to the channel length axis, the channel bolt must be released completely, inserted and tightened again.	

TABLE 12—HS / HZS / HSR CHANNEL BOLTS: INSTALLATION TORQUES

CRITERIA	SYMBOL	UNITS	POSITION OF FIXTURE	GRADE / MATERIAL	ANCHOR CHANNEL	CHANNEL BOLT SIZES M8	M10	M12	M16	M20	M24	M27	M30
Installation torque	T_{inst} [1]	lbf-ft. (Nm)	General (Fig. 4.1)	Steel 4.6 / 8.8 Stainless steel 50 / 70	28/15	5 (7)	9 (12)	12 (16)	-	-	-	-	-
					38/17	-	10 (14)	14 (19)	30 (40)	-	-	-	-
					41/22	-		15 (20)	30 (40)	-	-	-	-
					40/22	-	11 (15)	18 (25)	48 (65)	-	-	-	-
					40/22 with HSR channel bolts[2]	-	-	-	110 (150)	-	-	-	-
					50/30	-	11 (15)	18 (25)	48 (65)	85 (115)	-	-	-
					52/34	-	11 (15)	18 (25)	48 (65)	100 (135)	-	-	-
					55/42	-	11 (15)	18 (25)	48 (65)	100 (135)	170 (230)	-	-
					72/48	-	-	-	-	100 (135)	170 (230)	251 (340)	339 (460)
			Steel to steel contact (Figs. 4.2, 4.3)	Steel 4.6	28/15	5 (7)	10 (13)	13 (18)	-	-	-	-	-
					38/17	-	12 (16)	17 (23)	44 (60)	-	-	-	-
					40/22	-	12 (16)	21 (28)	48 (65)	-	-	-	-
					50/30	-	12 (16)	21 (28)	52 (70)	100 (135)	170 (230)	-	-
					72/48	-	-	-	-	100 (135)	170 (230)	251 (340)	339 (460)
				Steel 8.8	28/15	-	-	-	-	-	-	-	-
					38/17	-	-	48 (65)	100 (135)	-	-	-	-
					41/22	-	-	37 (50)	103 (140)	-	-	-	-
					40/22	-	30 (40)	55 (75)	136[2] (185)[2]	-	-	-	-
					50/30	-	30 (40)	55 (75)	136 (185)	266 (360)	461 (625)	-	-
					72/48	-	-	-	-	266 (360)	461 (625)	664 (900)	-
				Stainless steel 50	28/15	-	-	-	-	-	-	-	-
					38/17	-	-	-	33 (45)	-	-	-	-
					41/22	-	-	15 (20)	37 (50)	-	-	-	-
				Stainless steel 70	28/15	11 (15)	22 (30)	-	-	-	-	-	-
					38/17	-	22 (30)	30 (40)	-	-	-	-	-

T_{inst} must not be exceeded

[2] The tabulated value is also valid for HSR channel bolts.

TABLE 13— APPLICABLE SECTIONS OF THE IBC UNDER EACH EDITION OF THE IBC

2024 IBC	2021 IBC	2018 IBC	2015 IBC
Section 1605.1		Section 1605.2 or 1605.3	
Section 1705.1.1 and Table 1705.3			
Section 1901.3			
Sections 1903 and 1905			

TABLE 14— APPLICABLE SECTIONS OF ACI 318 UNDER EACH EDITION OF THE IBC

2024 IBC	2021 IBC	2018 IBC	2015 IBC
ACI 318-19		**ACI 318-14**	
5.3		5.3	
Chapter 17		Chapter 17	
17.5.1		17.3.1	
17.5.1.2		17.3.1	
Table 17.5.2		Table 17.3.1.1	
17.5.3		17.3.3	
17.6.3.1		17.4.3.1	
17.6.3.2		17.4.3.4	
17.6.3.3		17.4.3.6	
17.10		17.2.3	
17.10.5.2		17.2.3.4.2	
17.10.5.3 (b), (c) or (d)		17.2.3.4.3 (b), (c) or (d)	
17.10.6.2		17.2.3.5.2	
17.10.6.3		17.2.3.5.3	
24.3.2		24.3.2	
24.3.3		24.3.3	
Chapter 25		Chapter 25	
25.5		25.5	

ICC-ES Evaluation Report

ESR-1008 City of LA Supplement

Reissued April 2025
Revised April 24, 2025
This report is subject to renewal April 2026.

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DIVISION: 03 00 00—CONCRETE
Section: 03 15 19—Cast-In Concrete Anchors
Section: 03 16 00—Concrete Anchors

REPORT HOLDER:

LEVIAT GMBH

EVALUATION SUBJECT:

HALFEN HTA ANCHOR CHANNELS AND HS / HSR / HZS CHANNEL BOLTS

1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that the HALFEN HTA anchor channels and HALFEN HS / HSR / HZS channel bolts, described in ICC-ES evaluation report ESR-1008, have also been evaluated for compliance with the codes noted below, as adopted by the Los Angeles Department of Building and Safety (LADBS).

Applicable code editions:

- 2020 *City of Los Angeles Building Code* (LABC)
- 2020 *City of Los Angeles Residential Code* (LARC)

2.0 CONCLUSIONS

HALFEN HTA anchor channels and HS / HSR / HZS channel bolts, described in Sections 2.0 through 7.0 of evaluation report ESR-1008, comply with LABC Chapter 19, and the LARC, and are subject to the conditions of use described in this supplement.

3.0 CONDITIONS OF USE

The HALFEN HTA anchor channels and HALFEN HS / HSR / HZS channel bolts described in this evaluation report supplement must comply with all of the following conditions:

- All applicable sections in the evaluation report ESR-1008.
- The design, installation, conditions of use and identification of the anchoring systems are in accordance with the 2018 *International Building Code®* (IBC) provisions noted in the evaluation report ESR-1008.
- The design, installation and inspection are in accordance with additional requirements of LABC Chapters 16 and 17, as applicable.
- Under the LARC, an engineered design in accordance with LARC Section R301.1.3 must be submitted.
- The allowable and strength design values listed in the evaluation report and tables are for the connection of the anchoring systems to the concrete. The connection between the anchoring systems and the connected members shall be checked for capacity (which may govern).

This supplement expires concurrently with the evaluation report, reissued April 2025 and revised April 24, 2025.

ICC-ES Evaluation Report

ESR-1008 CA Supplement

w/ DSA and OSHPD

Reissued April 2025

Revised April 24, 2025

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Applicable code editions:

- 2019 *California Building Code* (CBC)

For evaluation of applicable chapters adopted by the California Office of Statewide Health Planning and Development (OSHPD) AKA: California Department of Health Care Access and Information (HCAI) and the Division of State Architects (DSA), see Sections 2.1.1 and 2.1.2 below.

- 2019 *California Residential Code* (CRC)

2.0 CONCLUSIONS

2.1 CBC:

The HALFEN HTA anchor channels and HALFEN HS / HSR / HZS channel bolts for use in cracked and uncracked concrete, described in Sections 2.0 through 7.0 of the evaluation report ESR-1008, comply with CBC Chapter 19, provided the design and installation are in accordance with the 2018 *International Building Code®* (IBC) provisions noted in the evaluation report and the additional design and inspection requirements of CBC Chapters 16 and 17, as applicable.

2.1.1 OSHPD:

The HALFEN HTA anchor channels and HALFEN HS / HSR / HZS channel bolts for use in cracked and uncracked concrete, described in Sections 2.0 through 7.0 of the evaluation report ESR-1008, comply with the CBC amended Chapter 19 [OSHPD 1R, 2 and 5] and Chapter 19A [OSHPD 1 and 4], provided the design and installation are in accordance with the 2018 *International Building Code®* (IBC) provisions noted in the evaluation report and the following conditions:

1. The design, installation and inspection are in accordance with the additional requirements of CBC Chapter 16 [OSHPD 3], amended Chapter 16 [OSHPD 1R, 2 and 5], Chapter 16A [OSHPD 1 and 4], amended Chapter 17 [OSHPD 1R, 2 and 5] and Chapter 17A [OSHPD 1 and 4]

2.1.2 DSA:

The HALFEN HTA anchor channels and HALFEN HS / HSR / HZS channel bolts for use in cracked and uncracked concrete, described in Sections 2.0 through 7.0 of the evaluation report ESR-1008, comply with the CBC amended Chapter 19 [DSA-SS/CC] and Chapter 19A [DSA-SS], provided the design and installation are in accordance with the 2018 *International Building Code®* (IBC) provisions noted in the evaluation report and the following conditions:

1. The design, installation and inspection are in accordance with the additional requirements of CBC amended Chapter 16 [DSA-SS/CC], Chapter 16A [DSA-SS], and Chapter 17A [DSA-SS, DSA-SS/CC].

2.2 CRC:

The HALFEN HTA anchor channels and HALFEN HS / HSR / HZS channel bolts for use in cracked and uncracked concrete, described in Sections 2.0 through 7.0 of the evaluation report ESR-1008, comply with CRC Sections R301.1.3, provided the design and installation are in accordance with the 2018 *International Building Code*® (IBC) provisions noted in the evaluation report and the additional design and inspection requirements of CBC Chapters 16 and 17, as applicable.

This supplement expires concurrently with the evaluation report, reissued April 2025 and revised April 24, 2025.